



Office of the Administrative Director – Financial Services Department

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Terri Gearon
FINANCIAL SERVICES DIRECTOR

May 24, 2019

MEMORANDUM

TO WHOM IT MAY CONCERN:

FROM: Terri Gearon, Financial Services Director /s/ Terri Gearon

SUBJECT: ADDENDUM NO. 2
JUDICIARY PROJECT NO. J19313
DESIGN, FURNISH, INSTALL, AND MAINTAIN A PHOTOVOLTAIC SYSTEM
WITH BATTERY STORAGE FOR THE RONALD TY MOON JUDICIARY
COMPLEX IN KAPOLEI

Transmitted herewith is a copy of Addendum No. 2 providing the following:

- **Amend Notice to Offerors, “All Other Questions” contact information to read as follows:**

All Other Questions:

Contact Person	Kelly Kimura
Contract's e-mail address	Kelly.Y.Kimura@courts.hawaii.gov

- **Include Foundation Investigation Kapolei Judiciary Complex, Kapolei, Oahu, Hawaii document as part of the RFP J19313.**
- **Include Kapolei Additional Drawings document as part of the RFP J19313.**

Should you have any technical questions regarding this Addendum, please call the Officer-In-Charge for this project Dee Dee Letts via phone at (808) 538-5990 or via email at deedee.d.letts@courts.hawaii.gov. Other questions may be directed to Kelly Kimura of the Judiciary Contracts & Purchasing Office via phone at (808) 538-5805, or via email Kelly.Y.Kimura@courts.hawaii.gov.

Foundation Investigation

Kapolei Judiciary Complex

Kapolei, Oahu, Hawaii

**FOUNDATION INVESTIGATION
KAPOLEI JUDICIARY COMPLEX
KAPOLEI, OAHU, HAWAII**

for

ARCHITECTS HAWAII, LTD.

HIRATA & ASSOCIATES, INC.
W.O. 05-4124
January 23, 2006

January 23, 2006
W.O. 05-4124

Mr. Michael Kim
Architects Hawaii, Ltd.
1001 Bishop Street, Suite 300
Honolulu, Hawaii 96813



Hirata & Associates

Geotechnical
Engineering

Hirata & Associates, Inc.

99-1433 Koaha Pl
Aiea, HI 96701
tel 808.486.0787
fax 808.486.0870

Dear Mr. Kim:

Our report, "Foundation Investigation, Kapolei Judiciary Complex, Kapolei, Oahu, Hawaii," dated January 23, 2006, our Work Order 05-4124 is enclosed. This investigation was conducted in general conformance with the scope of work presented in our proposal dated November 4, 2004.

The predominant soil encountered in our borings was tan coral rubblestone. Coral rubblestone consists of a conglomerate of coralline silt, sand, and coral fragments, oftentimes partially cemented by calcium carbonate. The coral rubblestone encountered in our borings was in a medium dense to dense condition, with occasional medium hard sections, and extended to the maximum depths drilled. Borings B4, B7, B8, and B11 through B16, encountered a thin surface layer of brown and grayish brown clayey silt. In addition, borings B3, and B5 through B8, encountered a layer of grayish brown silty clay at depths ranging from about 19 to 28 feet. Groundwater was not encountered in our borings.

We understand that the Family Court Center will have one level of basement parking and that the Juvenile Detention Center will have a below grade service area and sally port. Although finish elevations were not available at the time of this report, we expect that structural excavations will generally expose the medium dense to dense coral rubblestone. Conventional spread footings founded directly on the medium dense to dense coral rubblestone may be used to support the proposed structures.

The following is a summary of our geotechnical recommendations. This summary is not intended to be a substitute for our report which includes more detailed explanations of our recommendations, as well as additional requirements.

- Allowable bearing value = 4,000 psf
- Coefficient of friction = 0.4
- Passive earth pressure = 350 pcf

We appreciate this opportunity to be of service. Should you have any questions concerning this report, please feel free to call on us.

Very truly yours,

HIRATA & ASSOCIATES, INC.

Paul S. Morimoto

Vice President

PSM:RIKY:ph

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FOUNDATION INVESTIGATION

KAPOLEI JUDICIARY COMPLEX

KAPOLEI, OAHU, HAWAII

INTRODUCTION

This report presents the results of our foundation investigation performed for the proposed Kapolei Judiciary Complex in Kapolei, Oahu, Hawaii. Our work scope for this study included the following:

- A visual reconnaissance of the site and its vicinity to observe existing conditions which may affect the project. The general location of the project site is shown on the enclosed Location Map, Plate A2.1.
- A review of available in-house soils information pertinent to the site and the proposed project.
- Drilling and sampling sixteen exploratory borings to depths ranging from about 6 to 40.5 feet. A description of our field investigation is summarized on Plates A1.1 and A1.2. The approximate exploratory boring locations are shown on the enclosed Boring Location Plan, Plate A2.2, and the soils encountered in the borings are described on the Boring Logs, Plates A4.1 through A4.24.
- Laboratory testing of selected soil samples. Testing procedures are presented in the Description of Laboratory Testing, Plates B1.1 through B1.3. Test results are presented in the Description of Laboratory Testing, as well as on the Unified Soil Classification System chart (Plate A3.2), Boring Logs (Plates A4.1 through A4.24), Consolidation Test reports (Plates B2.1 through B2.5), Direct Shear Test reports (Plates B3.1 through B3.7), Modified Proctor Curve reports (Plates B4.1 and B4.2), CBR Stress Penetration Test Curve report (Plates B5.1), and Gradation Curve reports (Plates B6.1 and B6.2).
- Engineering analyses of the field and laboratory data.
- Preparation of this report presenting geotechnical recommendations for the design of foundations, seismic considerations, slabs-on-grade, resistance to lateral pressures, flexible and rigid pavement, and site grading.

PROJECT CONSIDERATIONS

The proposed Kapolei Judiciary Complex will consist of a Family Court Facility and a Juvenile Detention Facility. The Family Court Facility will be located in the northern portion of the site, while the Juvenile Detention Facility will be located in the southeastern section. The Family Court Facility will be a 4-story structure with below grade corridors connecting elevators serving the holding cells. The Juvenile Detention Facility will be two stories in height. The Juvenile Detention Facility and Family Court Facility will be connected at ground level by a sally port. We assume that reinforced concrete and concrete masonry construction will be used, and although not available at the time of this report, relatively heavy structural loads are anticipated.

The project will also include AC paved parking areas in the western portion of the site, and in a smaller area in the northeast, adjacent to the Family Court Facility.

Finish grades were not available at the time of this report. However, the site is relatively level and, except for the basement excavations, only minor site grading is expected in the remaining areas.

SITE CONDITIONS

The subject property is located southeast of the intersection between Kamokila Boulevard and Kapolei Parkway. Vacant land, which may have been previously cleared, borders the site on the west, while undeveloped overgrown land borders the site on the north and east. An abandoned railroad track parallels the south property line, just beyond the parcel.

The site is vacant of structures and, at the time of our exploratory fieldwork, was covered with light vegetation. However, the site appears to have been recently cleared.

Drainage over the site flows in a southerly to southeasterly direction. Total relief over the site is approximately 10 feet, with ground elevations ranging from about +68 in the northeast to about +58 in the southeast.

SOIL CONDITIONS

The predominant soil encountered in our borings consisted of tan coral rubblestone. Coral rubblestone consists of a conglomerate of coralline silt, sand, and coral fragments, oftentimes partially cemented by calcium carbonate. Typical of coral rubblestone deposits, the quantities of silt, sand, and coral fragments vary throughout the stratum. The coral rubblestone was in a medium dense to dense condition with occasional medium hard sections, and extended to the maximum depths drilled.

Borings B4, B7, B8, and B11 through B16, encountered a thin layer of brown to grayish brown clayey silt with coral gravel overlying the coral rubblestone. The surface clayey silt ranged in thickness from only about 1 to 2 feet.

A layer of grayish brown silty clay was encountered in borings B3, and B5 through B8, at depths ranging from approximately 19 to 28 feet. The silty clay layer was in a stiff to medium stiff condition, and ranged in thickness from about 3.5 to 15 feet. Reddish brown clayey silt was encountered in boring B9 between 5.5 and 10 feet. Laboratory testing on the silty clay and clayey silt resulted in relatively low expansion potentials.

Neither groundwater nor seepage water was encountered in the borings.

CONCLUSIONS AND RECOMMENDATIONS

Based on our exploratory fieldwork and laboratory testing, we believe that from a geotechnical viewpoint, the site can generally be developed as planned.

Finish elevations were not available at the time of this report. However, we expect that structural excavations will generally expose the medium dense to dense coral rubblestone. Conventional spread footings founded directly on coral rubblestone may be used to support the proposed structures. Building slabs-on-grade will require only the standard 4-inch gravel cushion and vapor barrier.

If exposed at the bottom of foundation excavations or at slab-on-grade subgrade elevations, the grayish brown silty clay should be overexcavated to a minimum 24 inches below the bottom of footing and slab elevations, and replaced with granular fill. The gravel cushion and base course layers recommended below slabs-on-grade may be considered part of the granular fill. The remainder of the granular fill section should consist of granular structural fill, such as the onsite coral rubblestone.

Foundations

Conventional spread footings founded directly on the medium dense to dense coral rubblestone may be used to support the proposed structures. Foundations may be designed for an allowable bearing value of 4,000 pounds per square foot. The recommended allowable bearing value is for the total of dead and frequently applied live loads, and may be increased by one-third for short duration loading which includes the effect of wind and seismic forces.

Footings should be a minimum 16 inches in width and embedded at least 24 inches below finish adjacent grade. The bottom of all footing excavations should be thoroughly tamped and cleaned of loose or deleterious material prior to placement of reinforcing steel and concrete.

Seismic Design

Based on the 1997 Uniform Building Code, the site is located within Seismic Zone 2A. Within this zone, a seismic zone factor (Z) equal to 0.15 is recommended (97 UBC Table 16-I) for calculation of shear and lateral load imparted on structures during an earthquake. Based on borings drilled as part of this study and our knowledge of the deep soil conditions in the area, the subsurface soils can be characterized as a dense soil profile. Therefore, soil profile type S_c is recommended for this site.

Lateral Design

Resistance to lateral loading may be provided by friction acting at the base of foundations and by passive earth pressure acting on the buried portions of foundations.

An allowable coefficient of friction of 0.4 may be used with the dead load forces. Passive earth pressure may be computed as an equivalent fluid having a density of 350 pounds per cubic foot with a maximum earth pressure of 3,500 pounds per square foot. Unless covered by pavement or concrete slabs, the upper 12 inches of soil should not be considered in computing lateral resistance.

For active earth pressure considerations, equivalent fluid pressures of 40 and 55 pounds per cubic foot may be used for freestanding and restrained conditions, respectively. To prevent buildup of hydrostatic pressures, weepholes or subdrains should be included in the design of any retaining structures.

Foundation Settlement

Structural loads were not available at the time of this report. However, neither excessive total nor differential settlement is anticipated for foundations bearing on

the medium dense to dense coral rubblestone. The final building loads should be forwarded to our office, when available, for review.

Slabs-on-Grade

To provide uniform support, building slabs-on-grade should be underlain by a 4-inch cushion of clean gravel, such as #3 Fine (ASTM C33 Size No. 67). Where vehicular or forklift loading is anticipated, the concrete slabs-on-grade should be underlain by a minimum 6 inches of aggregate base course. The base course is in lieu of the gravel cushion.

All slabs should also be protected by a vapor barrier placed over the cushion material or aggregate base course.

The subgrade soils should be scarified to a depth of six inches, moistened to slightly above optimum moisture content, and compacted to a minimum 95 percent compaction as determined by ASTM D 1557. The overlying gravel cushion should be compacted to a level surface using vibratory equipment. Aggregate base course should be compacted to a minimum 95 percent compaction as determined by ASTM D 1557.

As previously mentioned, if exposed at slab subgrade elevations, the grayish brown silty clay should be overexcavated to a minimum 24 inches below the bottom of slab elevation, and replaced with granular fill. The gravel cushion and base course layers may be considered part of the granular fill. The remainder of the granular fill section should consist of granular structural fill, such as the onsite coral rubblestone.

Slabs-on-grade which will receive floor covering, especially "hard" floor covering such as slate or marble, should include control joints saw-cut into the concrete slab. The purpose of this is to help reduce the potential for reflective cracking of the floor

covering due to shrinkage cracks in the concrete slab. Proper curing of the concrete slabs will help reduce shrinkage cracking.

Exterior slabs-on-grade and concrete walkways should be underlain by at least 4 inches of aggregate base course.

Pavement Design

Pavement design was based on medium traffic consisting primarily of passenger vehicles and light duty trucks, with occasional heavy trucks. Pavement design for driveways and parking areas may be designed based on the following sections:

Flexible Pavement

2.5" Asphaltic Concrete

6.0" Aggregate Base Course (minimum CBR = 85)

8.5" Total Thickness

Rigid Pavement

6.0" Portland Cement Concrete

6.0" Aggregate Base Course (minimum CBR = 85)

12.0" Total Thickness

The exposed subgrade should be scarified to a minimum depth of 6 inches, moisture conditioned to slightly above the optimum moisture content, and compacted to a minimum 95 percent compaction as determined by ASTM D 1557. The aggregate base course should also be compacted to a minimum 95 percent compaction as determined by ASTM D 1557.

Site Grading

Site Preparation - The project site should be cleared of all vegetation and other deleterious material. Prior to placement of fill, the exposed subgrade should be scarified to a minimum depth of 6 inches, moisture conditioned to slightly above the optimum moisture content, and compacted to a minimum 95 percent compaction as determined by ASTM D 1557.

Onsite Fill Materials - The onsite coral rubblestone may be used in structural fills and backfills, provided all coral fragments larger than 3 inches in maximum dimension are either crushed or removed. The surface clayey silt will be acceptable for reuse in non-building areas, such as parking lots, and in non-structural fills. Rock and coral fragments larger than 3 inches in maximum dimension should be removed from the clayey silt prior to compaction.

Imported Fill Materials - Imported structural fill should be well-graded, non-expansive granular material. Specifications for imported granular structural fill should indicate a maximum particle size of 3 inches, and state that between 8 and 20 percent of soil by weight shall pass the #200 sieve. In addition, the plasticity index (P.I.) of that portion of the soil passing the #40 sieve shall not be greater than 10. Granular structural fill should also have a minimum CBR value of 12 and a CBR expansion value less than 1.0 percent when tested in accordance with ASTM D 1883.

Compaction - Structural fill should be placed in horizontal lifts restricted to eight inches in loose thickness, and compacted to at least 95 percent compaction as determined by ASTM D 1557.

Fill placed in areas which slope steeper than 5H:1V should be continually benched as the fill is brought up in lifts.

Structural Excavations - Based on our exploratory borings, we believe that excavations into the onsite coral rubblestone can be accomplished using conventional excavating equipment. The sidewalls of temporary cuts into the coral rubblestone should be stable at gradients of 1H:1V or flatter. However, the contractor should be responsible for conforming to all OSHA safety standards for excavations.

Slope Gradients - Permanent cut and fill slopes may be designed for gradients of 2H:1V or flatter. Slopes should be planted as soon as practical upon completion of grading to reduce the potential for erosion damage.

ADDITIONAL SERVICES

We recommend that we perform a general review of the final design plans and specifications. This will allow us to verify that the foundation design and earthwork recommendations have been properly interpreted and implemented in the design plans and construction specifications.

For continuity, we recommend that we be retained during construction to (1) check footing excavations prior to placement of reinforcing steel and concrete, (2) review and/or perform laboratory testing on import borrow to determine its acceptability for use in compacted fills, (3) observe structural fill and backfill placement and perform compaction testing, and (4) provide geotechnical consultation as required. Our services during construction will allow us to verify that our recommendations are properly interpreted and included in construction, and if necessary, to make modifications to those recommendations, thereby reducing construction delays in the event subsurface conditions differ from those anticipated.

LIMITATIONS

The boring logs indicate the approximate subsurface soil conditions encountered only at those times and locations where our borings were made, and may not represent conditions at other times and locations.

This report was prepared specifically for Architects Hawaii, Ltd. and their sub-consultants for design of the proposed Kapolei Judiciary Complex in Kapolei, Hawaii. The boring logs, laboratory test results, and recommendations presented in

this report are for design purposes only, and are not intended for use in developing cost estimates by the contractor.

During construction, should subsurface conditions differ from those encountered in our borings, we should be advised immediately in order to re-evaluate our recommendations, and to revise or verify them in writing before proceeding with construction.

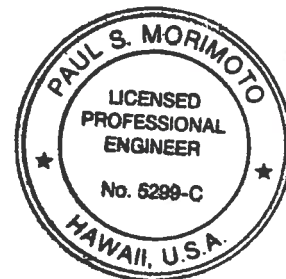
Our recommendations and conclusions are based upon the site materials observed, the preliminary design information made available, the data obtained from our site exploration, our engineering analyses, and our experience and engineering judgement. The conclusions and recommendations are professional opinions which we have strived to develop in a manner consistent with that level of care, skill, and competence ordinarily exercised by members of the profession in good standing, currently practicing under similar conditions. We will be responsible for those recommendations and conclusions, but will not be responsible for the interpretation by others of the information developed. No warranty is made regarding the services performed under this agreement, either express or implied.

Respectfully submitted,

HIRATA & ASSOCIATES, INC.


Rick Yoshida, Project Engineer


Paul S. Morimoto, Project Manager



This work was prepared by
me or under my supervision
Expiration Date of License:
April 30, 2006

APPENDIX A

FIELD INVESTIGATION

DESCRIPTION OF FIELD INVESTIGATION

GENERAL

The site was explored from July 5 to 11, 2005 by performing a visual site reconnaissance and drilling sixteen exploratory test borings ranging in depth from about 6 to 40.5 feet with a Mobile B40-L22 truck-mounted drill rig.

During drilling operations, the soils were continuously logged by our field engineer and classified by visual examination in accordance with the Unified Soil Classification System. The boring logs indicate the depths at which the soils or their characteristics change, although the change could actually be gradual. If the change occurred between sample locations, the depth was interpreted based on field observations. Classifications and sampling intervals are shown on the boring logs. A Boring Log Legend is presented on Plate A3.1. The Unified Soil Classification System is shown on Plate A3.2. The soils encountered are logged on Plates A4.1 through A4.24.

Boring locations were originally located in the field by M&E Pacific, Inc. based on a Site Plan provided by Architects Hawaii, Ltd., dated June 23, 2005. However, a revised Site Plan was received from Architects Hawaii, Ltd. on August 18, 2005, requiring the boring locations to be revised. The revised boring locations were then located by measuring/taping offsets from the original boring locations provided. Ground surface elevations at the boring locations were estimated using the Topographic Map provided by Architects Hawaii, Ltd. The accuracy of the boring locations shown on Plate A2.2 and the surface elevations shown on the boring logs are therefore approximate, in accordance with the field methods used.

SOIL SAMPLING

Representative and bulk soil samples were recovered from the borings for selected laboratory testing and analyses. Representative samples were recovered by driving

January 23, 2006

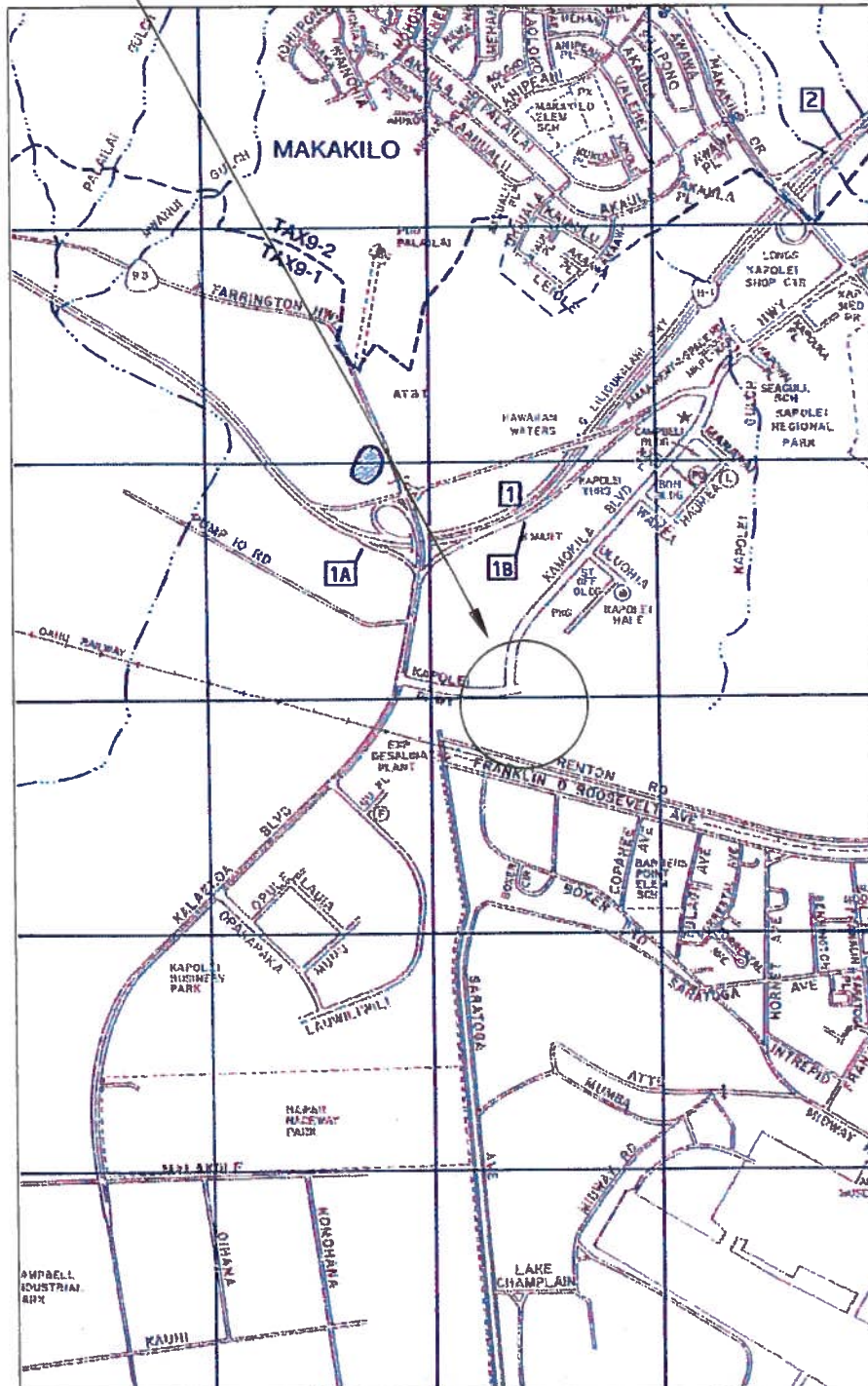
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Plate A1.2

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a 3-inch O.D. split tube sampler a total of 18 inches with a 140-pound hammer dropped from a height of 30 inches. The number of blows required to drive the sampler the final 12 inches are recorded at the appropriate depths on the boring logs, unless noted otherwise. Bulk soil samples were recovered from borings B7 and B12, between depths of about 0 to 2 and 1.5 feet, for selected laboratory testing that would aid in soil classification and pavement design.

PROJECT SITE



Reference: Bryan's Sectional Maps, 2003 Edition
(Copyright J.R. Clere, used with permission)



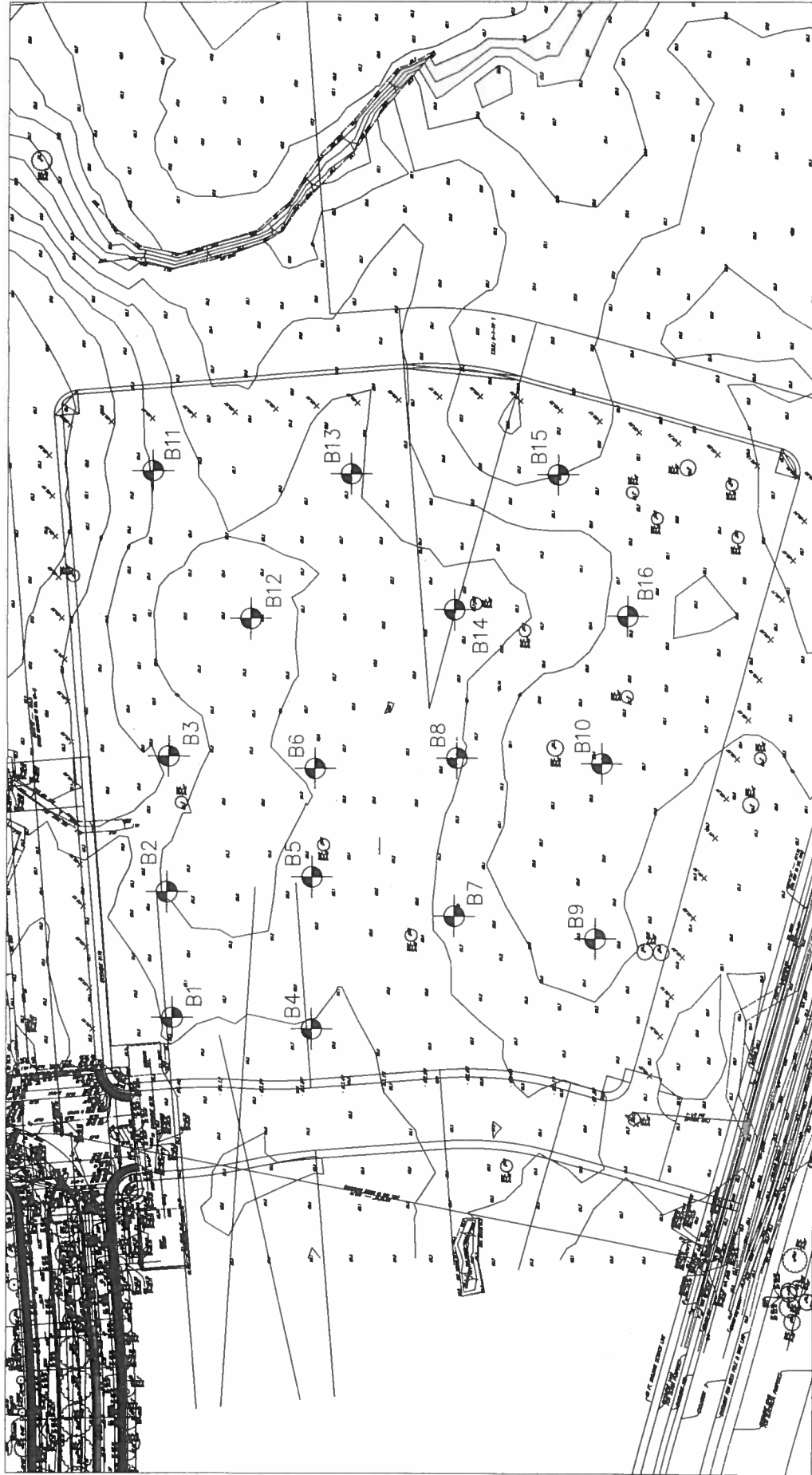
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Kapolei Judiciary Complex

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LOCATION MAP

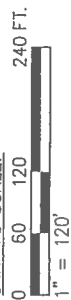
Plate A2.1



LEGEND:

— Approximate location of borings

GRAPHIC SCALE:






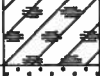
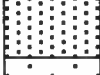

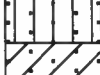






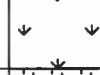


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Kapolei Judiciary Complex

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BORING LOCATION PLAN

Reference: Topographic Plan provided by Architects Hawaii, Ltd.

MAJOR DIVISIONS			GROUP SYMBOLS	TYPICAL NAMES
COARSE GRAINED SOILS (More than 50% of the material is LARGER than No. 200 sieve size.)	GRAVELS (More than 50% of coarse fraction is LARGER than the No. 4 sieve size.)	CLEAN GRAVELS (Little or no fines.)	 GW	Well graded gravels, gravel-sand mixtures, little or no fines.
			 GP	Poorly graded gravels or gravel-sand mixtures, little or no fines.
		GRAVELS WITH FINES (Appreciable amt. of fines.)	 GM	Silty gravels, gravel-sand-silt mixtures.
			 GC	Clayey gravels, gravel-sand-clay mixtures.
	SANDS (More than 50% of coarse fraction is SMALLER than the No. 4 sieve size.)	CLEAN SANDS (Little or no fines.)	 SW	Well graded sands, gravelly sands, little or no fines.
			 SP	Poorly graded sands or gravelly sands, little or no fines.
		SANDS WITH FINES (Appreciable amt. of fines.)	 SM	Silty sands, sand-silt mixtures.
			 SC	Clayey sands, sand-clay mixtures.
FINE GRAINED SOILS (More than 50% of the material is SMALLER than No. 200 sieve size.)	SILTS AND CLAYS (Liquid limit LESS than 50.)		 ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.
			 CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays.
			 OL	Organic silts and organic silty clays of low plasticity.
	SILTS AND CLAYS (Liquid limit GREATER than 50.)		 MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts.
			 CH	Inorganic clays of high plasticity, fat clays.
			 OH	Organic clays of medium to high plasticity, organic silts.
			HIGHLY ORGANIC SOILS	
			FRESH TO MODERATELY WEATHERED BASALT	
			VOLCANIC TUFF / HIGHLY TO COMPLETELY WEATHERED BASALT	
			CORAL	

SAMPLE DEFINITION		
 2" O.D. Standard Split Spoon Sampler	 Shelby Tube	RQD Rock Quality Designation
 3" O.D. Split Tube Sampler	 NX / 4" Coring	 Water Level

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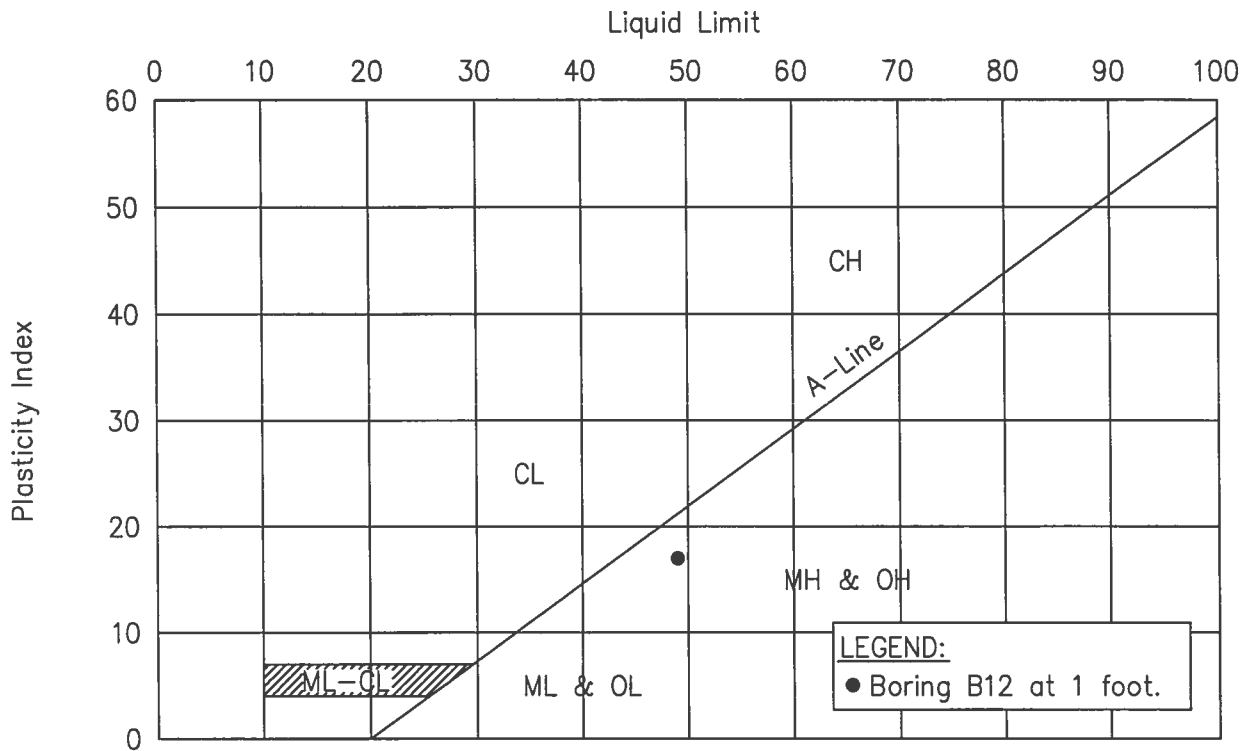
Kapolei Judiciary Complex

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BORING LOG LEGEND

Plate A3.1

PLASTICITY CHART



GRADATION CHART

COMPONENT DEFINITIONS BY GRADATION	
COMPONENT	SIZE RANGE
Boulders	Above 12 in.
Cobbles	3 in. to 12 in.
Gravel	3 in. to No. 4 (4.76 mm)
Coarse gravel	3 in. to 3/4 in.
Fine gravel	3/4 in. to No. 4 (4.76 mm)
Sand	No. 4 (4.76 mm) to No. 200 (0.074 mm)
Coarse sand	No. 4 (4.76 mm) to No. 10 (2.0 mm)
Medium sand	No. 10 (2.0 mm) to No. 40 (0.42 mm)
Fine sand	No. 40 (0.42 mm) to No. 200 (0.074 mm)
Silt and clay	Smaller than No. 200 (0.074 mm)

W.O. 05-4124

Kapolei Judiciary Complex

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UNIFIED SOIL CLASSIFICATION SYSTEM

Plate A3.2

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BORING LOG

W.O. 05-4124

BORING NO. B1 DRIVING WT. 140 lb. START DATE 7/06/05
 SURFACE ELEV. 64.2±* DROP 30 in. END DATE 7/07/05

DEPTH	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
0						Clayey SILT (MH) – Brown, moist, medium stiff, with coralline sand and gravel.
			10/No Penetration			CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
			40/3" No Recovery 10/No penetration			
5			50/5"	97	8	
10			49	101	18	
15			50/6"	106	12	
20			37	116	11	
25			24	87	9	Medium dense at 24 feet.
30			50	112	12	

* Elevations based on Topographic Survey Map provided by Architects Hawaii, Ltd.

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BORING LOG

W.O. 05-4124

BORING NO. B1 (continued) DRIVING WT. 140 lb. START DATE 7/06/05
 SURFACE ELEV. 64.2± DROP 30 in. END DATE 7/07/05

DEPTH	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
30						
35			50	105	4	
40			20/6" 10/No Penetration	105	9	
						End boring at 40 feet.
45						
						Neither groundwater nor seepage water encountered.
50						
55						
60						

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BORING LOG

W.O. 05-4124

BORING NO. B2 DRIVING WT. 140 lb. START DATE 7/05/05
 SURFACE ELEV. 61± DROP 30 in. END DATE 7/05/05

DEPTH FOOT	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
0						Clayey SILT (ML) – Brown, moist, medium stiff, with coralline sand and gravel.
						CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
			10/No penetration			
5			10/No penetration			Medium dense at 9 feet.
10			20	88	9	
15			46	94	13	Medium dense at 19 feet.
20			28	90	8	
25			49	110	13	
30			62	96	25	

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BORING LOG

W.O. 05-4124

BORING NO. B2 (continued) DRIVING WT. 140 lb. START DATE 7/05/05
 SURFACE ELEV. 61± DROP 30 in. END DATE 7/05/05

DEPTH	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
30						
35			50/5"	110	12	Grayish brown silty clay with coralline gravel at 34 feet.
40			57/6"	104	3	
						End boring at 39.5 feet.
45						
						Neither groundwater nor seepage water encountered.
50						
55						
60						

HIRATA & ASSOCIATES, INC.

BORING LOG

W.O. 05-4124

BORING NO. B3 DRIVING WT. 140 lb. START DATE 7/07/05
 SURFACE ELEV. 68.5± DROP 30 in. END DATE 7/07/05

DEPTH FOOT	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
0						CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
			31	91	15	
5			33	96	14	
10			53/9" 10/No Penetration	108	14	
15			32	99	9	
						Silty CLAY (CH) – Grayish brown, moist, medium stiff, with coralline gravel.
			46	102	9	
20						
25			26	93	18	
			72	94	25	
30						Stiff at 28 feet.

Plate A4.5

HIRATA & ASSOCIATES, INC.

BORING LOG

W.O. 05-4124

BORING NO. B3 (continued) DRIVING WT. 140 lb. START DATE 7/07/05
 SURFACE ELEV. 68.5± DROP 30 in. END DATE 7/07/05

DEPTH	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
30						
35			60	104	5	CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
			10/No Penetration			
40						End boring at 38 feet.
45						Neither groundwater nor seepage water encountered.
50						
55						
60						

HIRATA & ASSOCIATES, INC.

BORING LOG

W.O. 05-4124

BORING NO. B4 DRIVING WT. 140 lb. START DATE 7/06/05
 SURFACE ELEV. 65.1± DROP 30 in. END DATE 7/06/05

DEPTH	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
0						Clayey SILT (ML) – Grayish brown, moist, medium stiff.
						CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
5			50/6"	No Recovery		CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
						CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
10			38	104	13	CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
						CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
15			32	87	9	CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
						CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
20			27	97	7	CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
						CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
25			24	100	8	CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
						CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
30			45	83	41	CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)

HIRATA & ASSOCIATES, INC.

BORING LOG

W.O. 05-4124

BORING NO. B4 (continued) DRIVING WT. 140 lb. START DATE 7/06/05
 SURFACE ELEV. 65.1± DROP 30 in. END DATE 7/06/05

DEPTH	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
30						
			12	92	6	Medium dense at 33 feet.
35						
			15/No Penetration			
						End boring at 38 feet.
40						
45						
						Neither groundwater nor seepage water encountered.
50						
55						
60						

HIRATA & ASSOCIATES, INC.

BORING LOG

W.O. 05-4124

BORING NO. B5 DRIVING WT. 140 lb. START DATE 7/07/05
 SURFACE ELEV. 62.6± DROP 30 in. END DATE 7/07/05

DEPTH H O	GRAPH	S A M P L E	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
0						CORAL RUBBLESTONE – Tan, moist, medium dense. (Silty sand with coralline gravel)
		<input type="checkbox"/>	13	67	16	
		<input type="checkbox"/>	9	81	9	Loose at 3 feet.
5		<input type="checkbox"/>	30	102	2	
		<input type="checkbox"/>	15	96	4	
10						
		<input type="checkbox"/>	54	109	8	Dense from 14 feet.
15						
20		<input type="checkbox"/>	33	63	61	Silty CLAY (CH) – Grayish brown, moist, stiff, with coralline gravel.
		<input type="checkbox"/>	22	70	42	Medium stiff at 24 feet.
25						
						CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
		<input type="checkbox"/>	50/6"	99	19	
30						

HIRATA & ASSOCIATES, INC.

BORING LOG

W.O. 05-4124

BORING NO. B5 (continued) DRIVING WT. 140 lb. START DATE 7/07/05
 SURFACE ELEV. 62.6± DROP 30 in. END DATE 7/07/05

DEPTH	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
30						
35			34/6" 10/No penetration	118	4	
			10/No penetration			
40						End boring at 39 feet.
45						Neither groundwater nor seepage water encountered.
50						
55						
60						

HIRATA & ASSOCIATES, INC.

BORING LOG

W.O. 05-4124

BORING NO. B6 DRIVING WT. 140 lb. START DATE 7/06/05
 SURFACE ELEV. 61.9± DROP 30 in. END DATE 7/06/05

DEPTH	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
0						CORAL RUBBLESTONE – Tan, moist, medium dense. (Silty sand with coralline gravel)
		<input type="checkbox"/>	27	73	15	
		<input type="checkbox"/>	17	74	31	
5		<input type="checkbox"/>	47	96	10	
						Dense from 5 feet.
		<input type="checkbox"/>	50/7"	99	5	
10			10/No Penetration			
		<input type="checkbox"/>	60	91	13	
15						
		<input type="checkbox"/>	13/6"	90	22	
20			13/6"			
		<input type="checkbox"/>	16	74	38	Silty CLAY (CH) – Grayish brown, moist, medium stiff, with coralline gravel.
25						
		<input type="checkbox"/>	27	63	68	
30						

HIRATA & ASSOCIATES, INC.

BORING LOG

W.O. 05-4124

BORING NO. B6 (continued) DRIVING WT. 140 lb. START DATE 7/06/05
 SURFACE ELEV. 61.9± DROP 30 in. END DATE 7/06/05

DEPTH	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
30						
35			15/6"	70	41	
			14/6"			
40			30	86	7	
45						
50						
55						
60						

End boring at 40.5 feet.

Neither groundwater nor seepage water encountered.

HIRATA & ASSOCIATES, INC.

BORING LOG

W.O. 05-4124

BORING NO. B7 DRIVING WT. 140 lb. START DATE 7/08/05
SURFACE ELEV. 61.8± DROP 30 in. END DATE 7/08/05

DEPTH FOOT	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
0						Clayey SILT (ML) – Grayish brown, slightly moist, medium stiff, with sand and coralline gravel.
5			61/6"	No Recovery		CORAL RUBBLESTONE– Tan, moist, dense. (Silty sand with coralline gravel)
			57	98	14	
10			56			
15			38	94	6	
20			31			
25			36	98	16	Siltier at 23 feet.
30			30			Silty CLAY (CH) – Grayish brown, moist, stiff, with coralline gravel.

Plate A4.13

Plate A4.13

HIRATA & ASSOCIATES, INC.

BORING LOG

W.O. 05-4124

BORING NO. B7 (continued) DRIVING WT. 140 lb. START DATE 7/08/05
 SURFACE ELEV. 61.8± DROP 30 in. END DATE 7/08/05

DEPTH	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
30						
35			34/10" 10/No penetration	90	7	CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
			10/No penetration			
40						End boring at 38 feet.
45						Neither groundwater nor seepage water encountered.
50						
55						
60						

HIRATA & ASSOCIATES, INC.

BORING LOG

W.O. 05-4124

BORING NO. B8 DRIVING WT. 140 lb. START DATE 7/08/05
 SURFACE ELEV. 61.1± DROP 30 in. END DATE 7/08/05

DEPTH	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
0						Clayey SILT (ML) – Brown, slightly moist, medium stiff, with coralline gravel.
		<input type="checkbox"/>	43			CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
		<input type="checkbox"/>	58			
5						
		<input type="checkbox"/>	28	88	7	
10						
		<input type="checkbox"/>	32/3" 10/No Penetration			Medium dense at 8 feet.
15						
		<input type="checkbox"/>	34	99	17	
20						
		<input type="checkbox"/>	34			
25						Silty CLAY (CH) – Grayish brown, moist, stiff, with coralline gravel.
		<input type="checkbox"/>	41	84	28	CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
30						

HIRATA & ASSOCIATES, INC.

BORING LOG

W.O. 05-4124

BORING NO. B8 (continued) DRIVING WT. 140 lb. START DATE 7/08/05
SURFACE ELEV. 61.1± DROP 30 in. END DATE 7/08/05

DEPTH	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
30						
35			24/6" 10/No Penetration			
			25	95	5	
40						End boring at 39.5 feet.
45						Neither groundwater nor seepage water encountered.
50						
55						
60						

HIRATA & ASSOCIATES, INC.

BORING LOG

W.O. 05-4124

BORING NO. B9 DRIVING WT. 140 lb. START DATE 7/11/05
 SURFACE ELEV. 59.7± DROP 30 in. END DATE 7/11/05

DEPTH FOOT	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
0						CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
			10/No Penetration			CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
5			26	83	10	Clayey SILT (MH) – Reddish brown, moist, stiff, with coralline gravel.
			29/6"	82	31	CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
10						
			10/No Penetration			CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
15						CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
			33	108	6	End boring at 19.5 feet.
20						End boring at 19.5 feet.
						Neither groundwater nor seepage water encountered.
25						Neither groundwater nor seepage water encountered.
						Neither groundwater nor seepage water encountered.
30						Neither groundwater nor seepage water encountered.

HIRATA & ASSOCIATES, INC.

BORING LOG

W.O. 05-4124

BORING NO. B10 DRIVING WT. 140 lb. START DATE 7/11/05
 SURFACE ELEV. 58.6± DROP 30 in. END DATE 7/11/05

DEPTH FOOT	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
0						CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
		<input type="checkbox"/>	67	101	4	
		<input type="checkbox"/>	96/9"	No Recovery		
5		<input type="checkbox"/>	76	98	4	
10		<input type="checkbox"/>	51	109	5	
15		<input type="checkbox"/>	72	87	5	
20		<input type="checkbox"/>	31	100	13	
						End boring at 20.5 feet.
						Neither groundwater nor seepage water encountered.
25						
30						

HIRATA & ASSOCIATES, INC.

BORING LOG

W.O. 05-4124

BORING NO. B11 DRIVING WT. 140 lb. START DATE 7/05/05
 SURFACE ELEV. 64.5± DROP 30 in. END DATE 7/05/05

DEPTH	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
0						Clayey SILT (ML) – Brown, slightly moist, medium stiff.
			10/No Penetration			CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
			50/6"	92	13	
5			80/3"	91	13	
			10/No Penetration			
10			43	102	18	
						End boring at 10.5 feet.
15						Neither groundwater nor seepage water encountered.
20						
25						
30						

HIRATA & ASSOCIATES, INC.

BORING LOG

W.O. 05-4124

BORING NO. B12 DRIVING WT. 140 lb. START DATE 7/05/05
 SURFACE ELEV. 59.8± DROP 30 in. END DATE 7/05/05

DEPTH	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
0			25/2"	72	32	Clayey SILT (ML) – Brown, moist, medium stiff, with coralline gravel.
			10/No Penetration			CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
			57	102	12	
5			55/6"	96	12	Grayish brown silty clay at 5 feet.
						End boring at 6 feet.
10						
15						
20						
25						
30						

HIRATA & ASSOCIATES, INC.

BORING LOG

W.O. 05-4124

BORING NO. B13 DRIVING WT. 140 lb. START DATE 7/05/05
 SURFACE ELEV. 62.4± DROP 30 in. END DATE 7/05/05

DEPTH	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
0						
			10/6"	94	7	Silty SAND (SM) – Tan, slightly moist, dense, with coralline gravel.
			75/5"			
			81/9"	72	23	Silty CLAY (CH) – Grayish brown, moist, stiff, with coralline gravel.
			10/No Penetration			
5			80/8"	83	21	
			10/No Penetration			End boring at 6 feet.
10						
15						
20						
25						
30						

Neither groundwater nor seepage water encountered.

HIRATA & ASSOCIATES, INC.

BORING LOG

W.O. 05-4124

BORING NO. B14 DRIVING WT. 140 lb. START DATE 7/11/05
 SURFACE ELEV. 63.5± DROP 30 in. END DATE 7/11/05

DEPTH	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
0						Clayey SILT (ML) – Brown, moist, medium stiff, with coralline gravel.
		<input type="checkbox"/>	76	73	16	CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel) Medium dense at 3 feet.
		<input type="checkbox"/>	16	89	16	
5		<input type="checkbox"/>	32	93	6	
		<input type="checkbox"/>	59/11" 10/No Penetration	100	5	
		<input type="checkbox"/>	30	104	8	
15						
		<input type="checkbox"/>	37	107	16	
20						End boring at 20.5 feet.
25						Neither groundwater nor seepage water encountered.
30						

HIRATA & ASSOCIATES, INC.

BORING LOG

W.O. 05-4124

BORING NO. B15 DRIVING WT. 140 lb. START DATE 7/05/05
 SURFACE ELEV. 57.9± DROP 30 in. END DATE 7/05/05

DEPTH	GRAPH	SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
0			50/6" 10/No Penetration	80	20	Clayey SILT (ML) – Reddish brown, moist, medium stiff, with coralline gravel.
			95	109	7	CORAL RUBBLESTONE – Tan, moist, dense. (Silty sand with coralline gravel)
5			60/6" 10/No Penetration	95	8	
10			79	108	15	
						End boring at 10.5 feet.
15						Neither groundwater nor seepage water encountered.
25						
30						

HIRATA & ASSOCIATES, INC.

BORING LOG

W.O. 05-4124

BORING NO. B16 DRIVING WT. 140 lb. START DATE 7/11/05
 SURFACE ELEV. 58.8± DROP 30 in. END DATE 7/11/05

DEPTH FOOT	GRAPH			SAMPLE	BLOWS PER FOOT	DRY DENSITY (PCF)	MOIST. CONT. (%)	DESCRIPTION
0								Clayey SILT (ML) – Brown, slightly moist, medium stiff.
					21	87	12	CORAL RUBBLESTONE– Tan, moist, medium dense. (Silty sand with coralline gravel) Dense from 4 feet.
					49	98	18	
5								
10					50/3" 10/No Penetration	No Recovery		
					58	103	3	
15								End boring at 18 feet. <

APPENDIX B

LABORATORY TESTING

DESCRIPTION OF LABORATORY TESTING

CLASSIFICATION

Field classification was verified in the laboratory in accordance with the Unified Soil Classification System. Laboratory classification was determined by visual examination, Atterberg Limit tests performed in general accordance with ASTM D 4318, and sieve analysis testing performed in general accordance with ASTM D 422. Results of Atterberg Limit tests are plotted on Plate A3.2, while gradation test results are presented on Plates B6.1 and B6.2. The final classifications are shown at the appropriate locations on the Boring Logs, Plates A4.1 through A4.24.

MOISTURE-DENSITY

Representative samples were tested for field moisture content and dry unit weight. The dry unit weight was determined in pounds per cubic foot while the moisture content was determined as a percentage of dry weight. Samples were obtained using a 3-inch O.D. split tube sampler. Test results are shown at the appropriate depths on the Boring Logs, Plates A4.1 through A4.24.

CONSOLIDATION

Representative samples were tested for their consolidation characteristics. Test samples were 2.42 inches in diameter and 1 inch high. Porous stones were placed in contact with the top and bottom of test samples to permit addition and release of pore fluid. Loads were then applied in several increments in a geometric progression, and the resulting deformations recorded at selected time intervals. Test results are plotted on the Consolidation Test Reports, Plates B2.1 through B2.5.

SHEAR TESTS

Shear tests were performed in the Direct Shear Machine which is of the strain control type. The rate of deformation was approximately 0.02 inches per minute. Each sample was sheared under varying confining loads in order to determine the

Coulomb shear strength parameters, cohesion and angle of internal friction. Eighty percent of the maximum value was taken to determine the shear strength parameters. Test results are presented on Plates B3.1 through B3.7.

SWELL TESTS

Swell tests were performed on representative soil samples by first placing a 90 psf surcharge load on one inch high specimens. The samples were inundated with water, and total expansion was recorded after a period of at least 24 hours. Test results were recorded as a percentage of original height. Test results are summarized in the following table:

Sample	Sample Type	Recorded Expansion	Moisture Content Prior to Test
B3@28'	Representative	2.5%	25%
B5@19'	Representative	0.1%	61%

PROCTOR TEST

Modified Proctor tests were performed on bulk samples, obtained from borings B7 and B12 between depths of about 0 to 2 and 1.5 feet, in general accordance with ASTM D 1557. The test is used to determine the optimum moisture content at which the soil compacts to 100 percent density. Results are shown on Plates B4.1 and B4.2.

CALIFORNIA BEARING RATIO TEST

A CBR test was performed on a bulk sample, obtained from boring B7 between depths of about 0 to 2 feet, in general accordance with ASTM D 1883. The test is used to evaluate the relative quality of subgrade soils to be used in the design of flexible pavements. Results are shown on Plate B5.1.

January 23, 2006

W.O. 05-4124

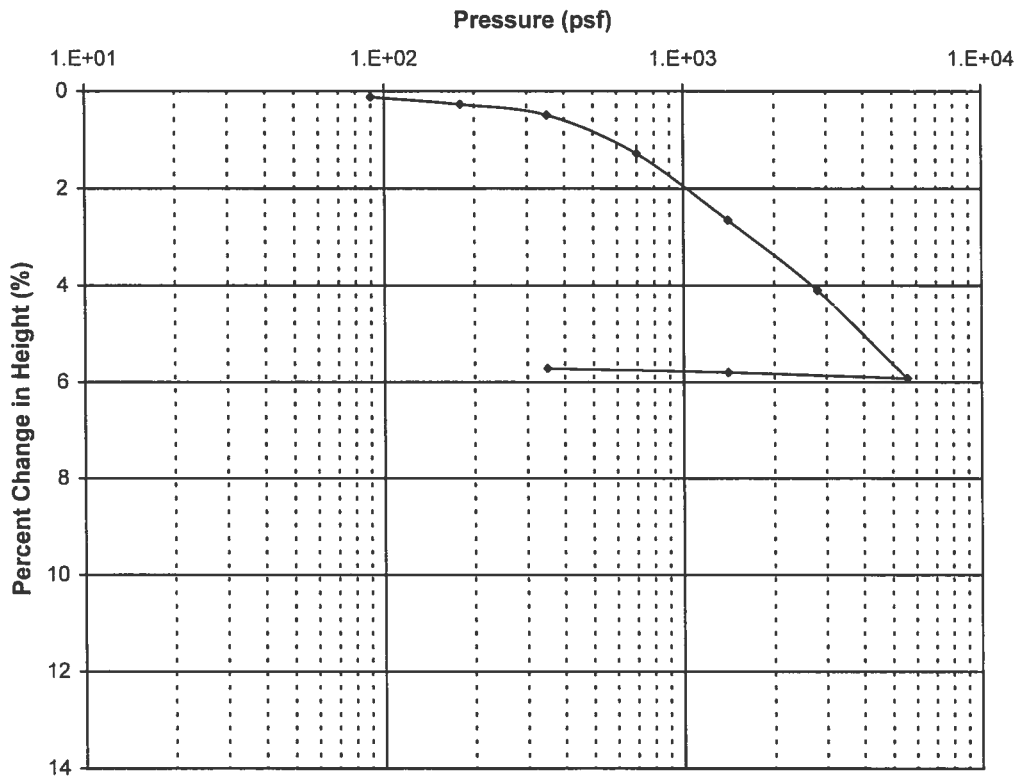
Plate B1.3

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SIEVE ANALYSIS

Sieve analysis tests were conducted on representative samples in general accordance with ASTM D 422. The test is used to classify granular soils. Test results are presented on Plates B6.1 and B6.2.

Consolidation Test Results



Sample Description

Boring No.:	B2	Depth (ft):	19
Soil Description:	Tan coral rubblestone		

	Moisture Content (%)	Dry Density (pcf)
Initial	8.0	90.1
Final	1.4	95.6

Remark: 8/27/05

W.O. 05-4124

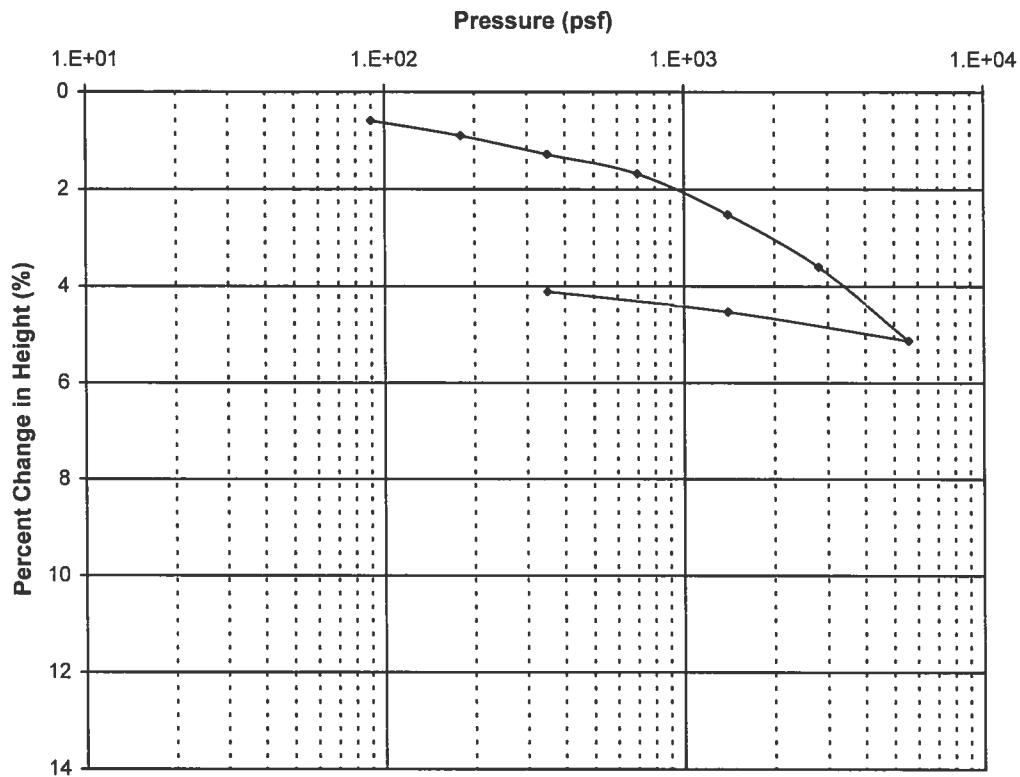
Kapolei Judiciary Complex

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CONSOLIDATION TEST

Plate B2.1

Consolidation Test Results



Sample Description

Boring No.: B3 **Depth (ft):** 23
Soil Description: Grayish brown silty clay

	Moisture Content (%)	Dry Density (pcf)
Initial	17.9	93.3
Final	12.4	97.3

Remark: 12/16/05

W.O. 05-4124

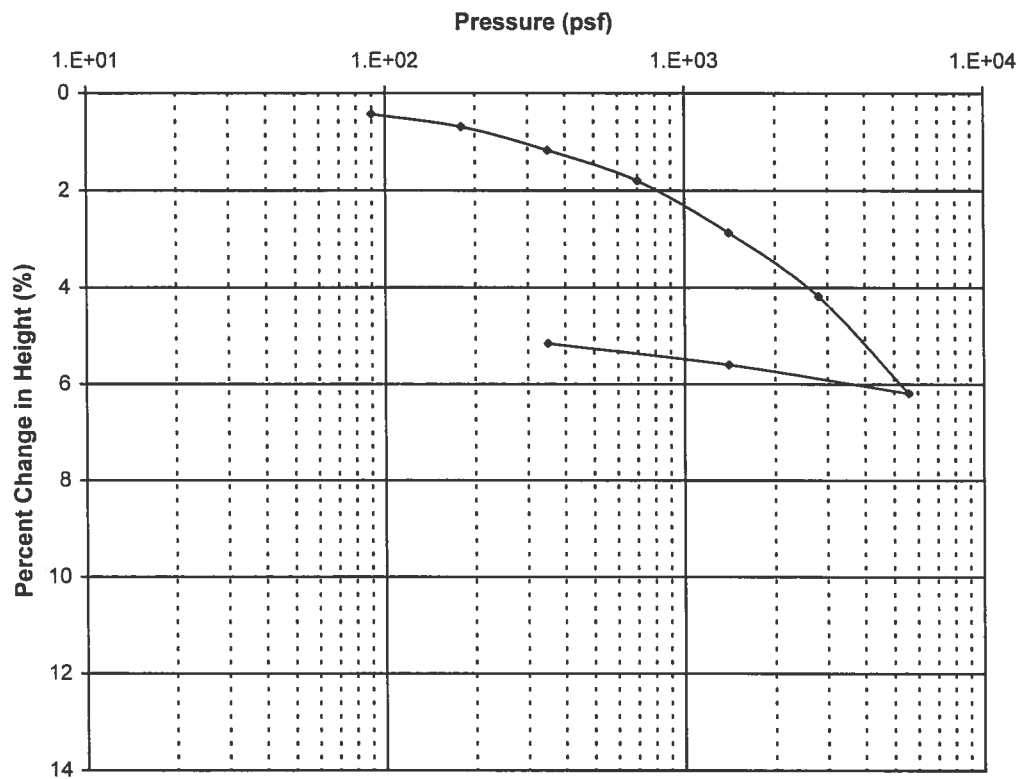
Kapolei Judiciary Complex

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CONSOLIDATION TEST

Plate B2.2

Consolidation Test Results



Sample Description

Boring No.: B6 Depth (ft): 24
 Soil Description: Grayish brown silty clay

	Moisture Content (%)	Dry Density (pcf)
Initial	38.4	74.0
Final	30.6	78.1

Remark: 12/16/05

W.O. 05-4124

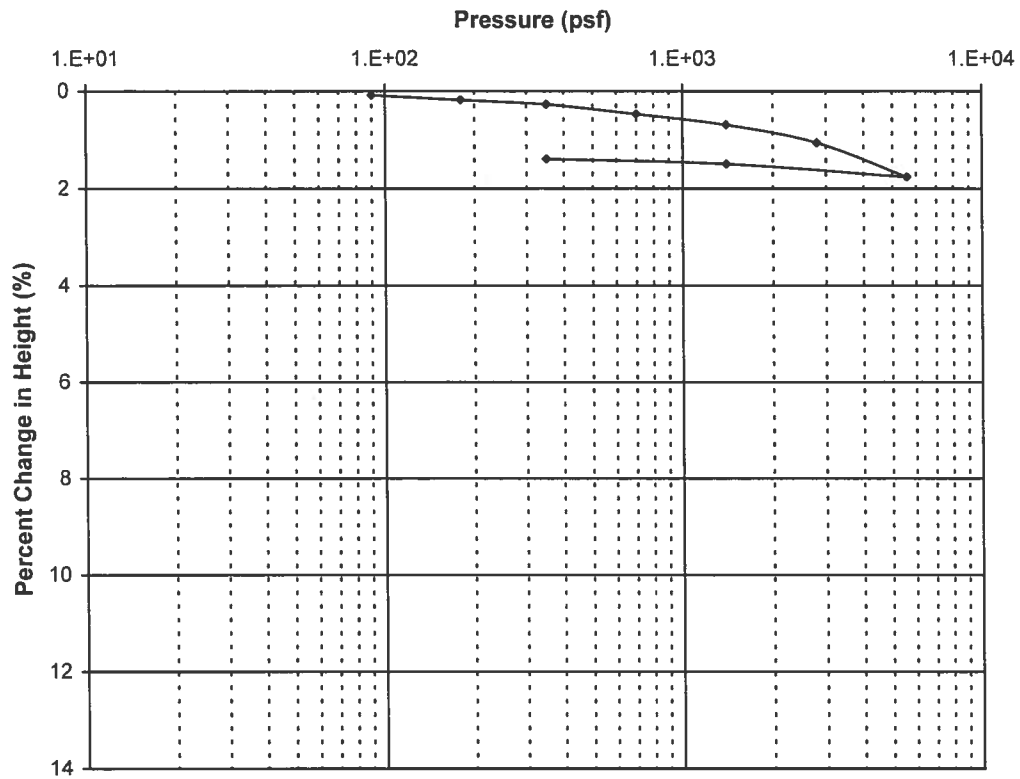
Kapolei Judiciary Complex

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CONSOLIDATION TEST

Plate B2.3

Consolidation Test Results



Sample Description

Boring No.: B8 **Depth (ft):** 18
Soil Description: Tan coral rubblestone

	Moisture Content (%)	Dry Density (pcf)
Initial	17.1	99.0
Final	5.0	100.4

Remark: 12/16/05

W.O. 05-4124

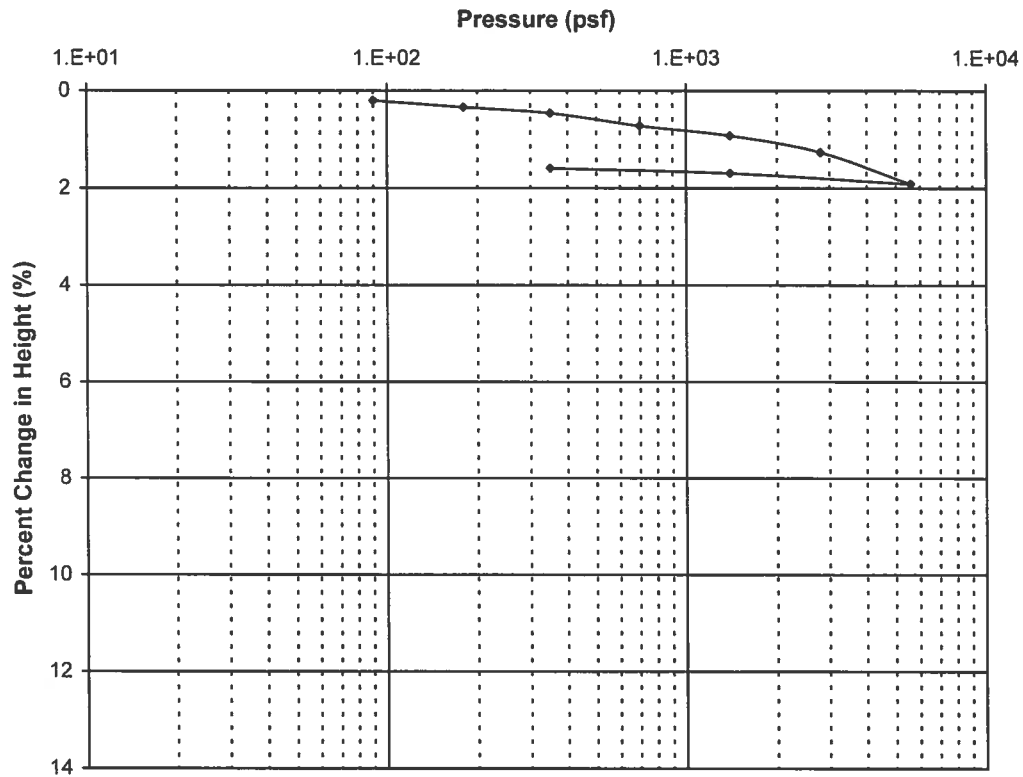
Kapolei Judiciary Complex

Hirata & Associates, Inc.

CONSOLIDATION TEST

Plate B2.4

Consolidation Test Results



Sample Description

Boring No.: B14 Depth (ft): 14
 Soil Description: Tan coral rubblestone

	Moisture Content (%)	Dry Density (pcf)
Initial	8.3	103.9
Final	7.2	105.6

Remark: 12/16/05

W.O. 05-4124

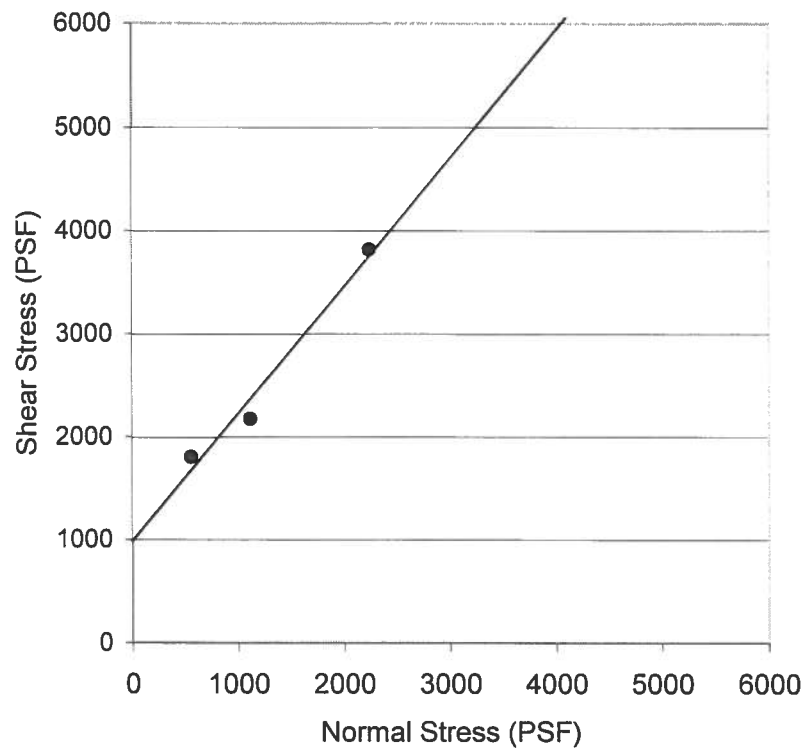
Kapolei Judiciary Complex

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CONSOLIDATION TEST

Plate B2.5

Direct Shear Test Results



Sample Description

Boring No.: B3 Depth (ft): 8
 Soil Description: Tan coral rubblestone
 Strength Intercept (C): 986.6 PSF (Peak Strength)
 Friction Angle (ϕ): 51.0 DEG (Peak Strength)

Remark: 12/6/05

W.O. 05-4124

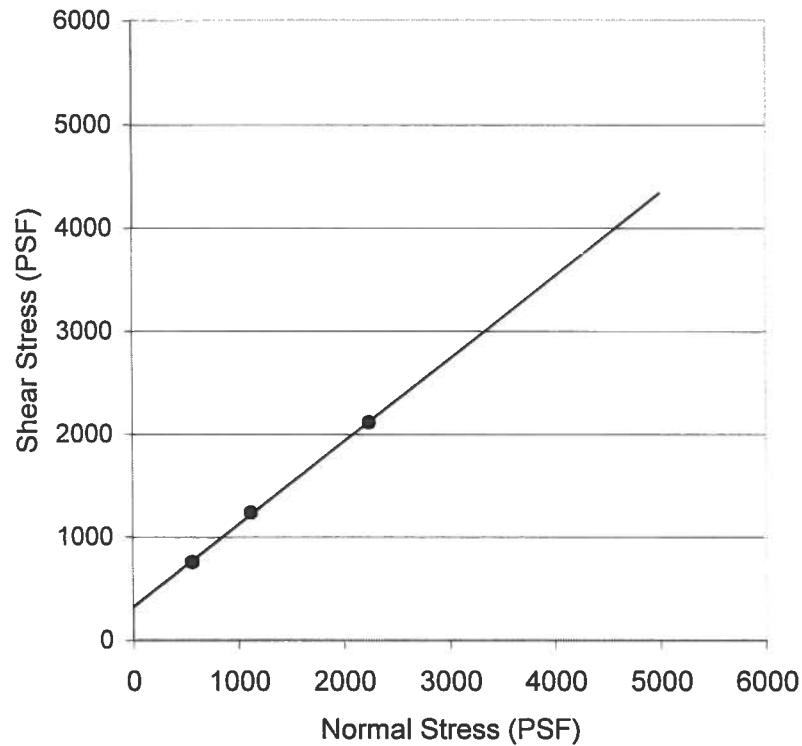
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DIRECT SHEAR TEST

Plate B3.1

Direct Shear Test Results



Sample Description

Boring No.: B5 Depth (ft): 5
 Soil Description: Tan coral rubblestone
 Strength Intercept (C): 320.3 PSF (Peak Strength)
 Friction Angle (ϕ): 38.8 DEG (Peak Strength)

Remark: 12/12/05

W.O. 05-4124

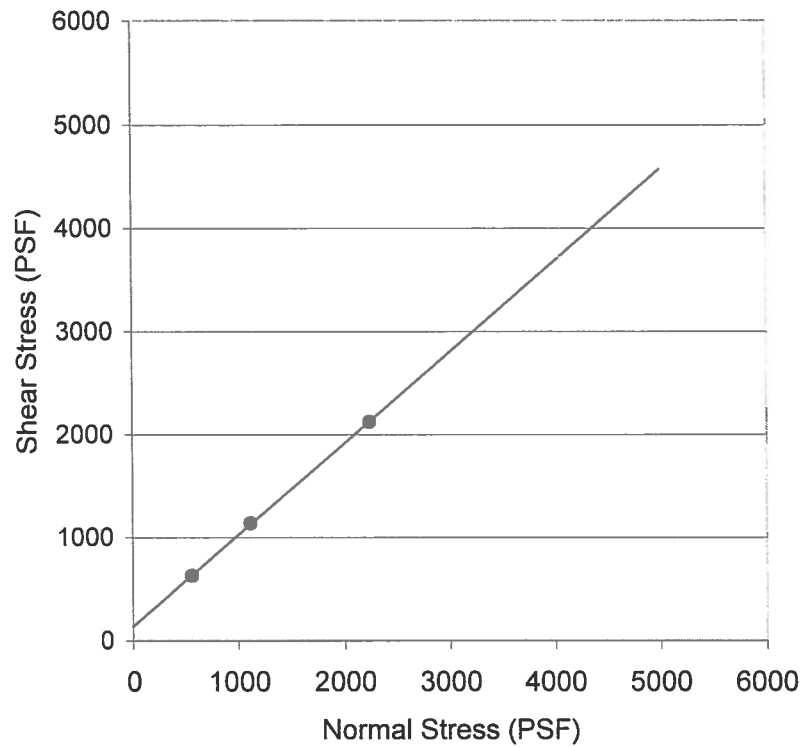
Kapolei Judiciary Complex

Hirata & Associates, Inc.

DIRECT SHEAR TEST

Plate B3.2

Direct Shear Test Results



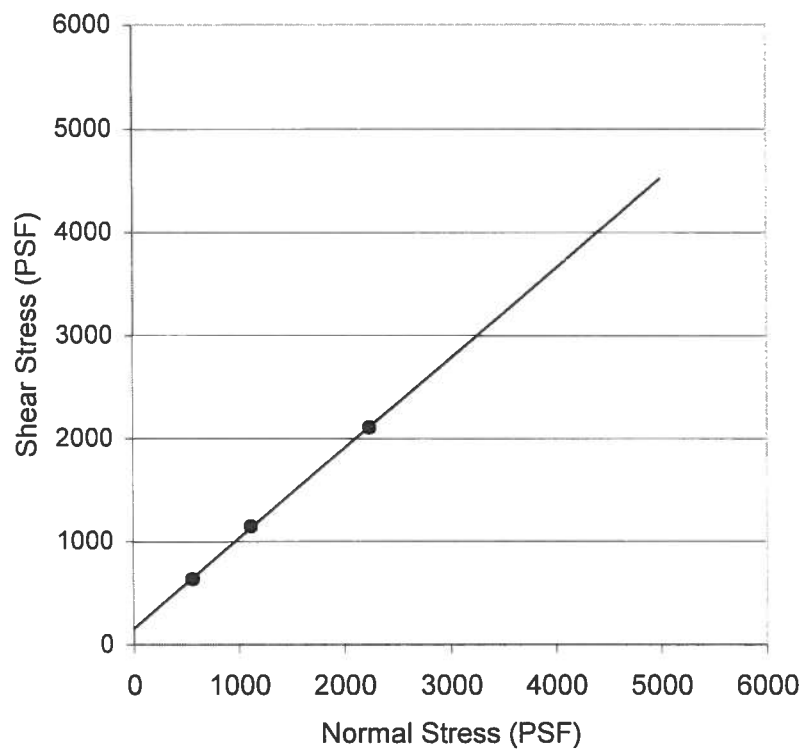
Sample Description

Boring No.:	B8	Depth (ft):	8
Soil Description:	Tan coral rubblestone		
Strength Intercept (C):	137.6 PSF		(Peak Strength)
Friction Angle (ϕ):	41.6 DEG		(Peak Strength)

Remark: 12/14/05

W.O. 05-4124	Kapolei Judiciary Complex
Hirata & Associates, Inc.	DIRECT SHEAR TEST

Direct Shear Test Results



Sample Description

Boring No.: B9 Depth (ft): 4
 Soil Description: Tan coral rubblestone
 Strength Intercept (C): 156.6 PSF (Peak Strength)
 Friction Angle (ϕ): 41.1 DEG (Peak Strength)

Remark: 12/13/05

W.O. 05-4124

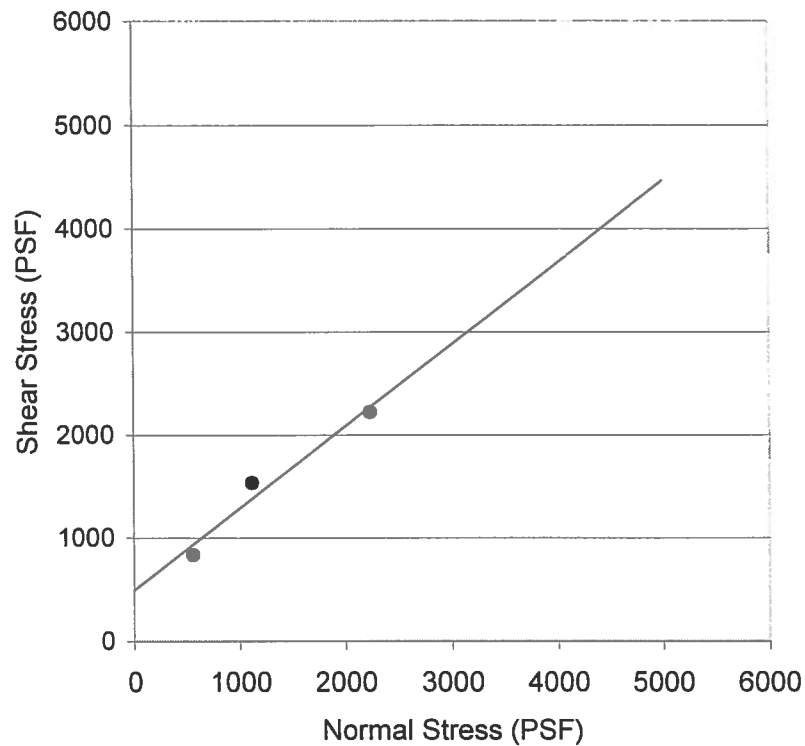
Kapolei Judiciary Complex

Hirata & Associates, Inc.

DIRECT SHEAR TEST

Plate B3.4

Direct Shear Test Results



Sample Description

Boring No.: B12 Depth (ft): 5
 Soil Description: Tan coral rubblestone
 Strength Intercept (C): 491.7 PSF (Peak Strength)
 Friction Angle (ϕ): 38.5 DEG (Peak Strength)

Remark: 12/9/05

W.O. 05-4124

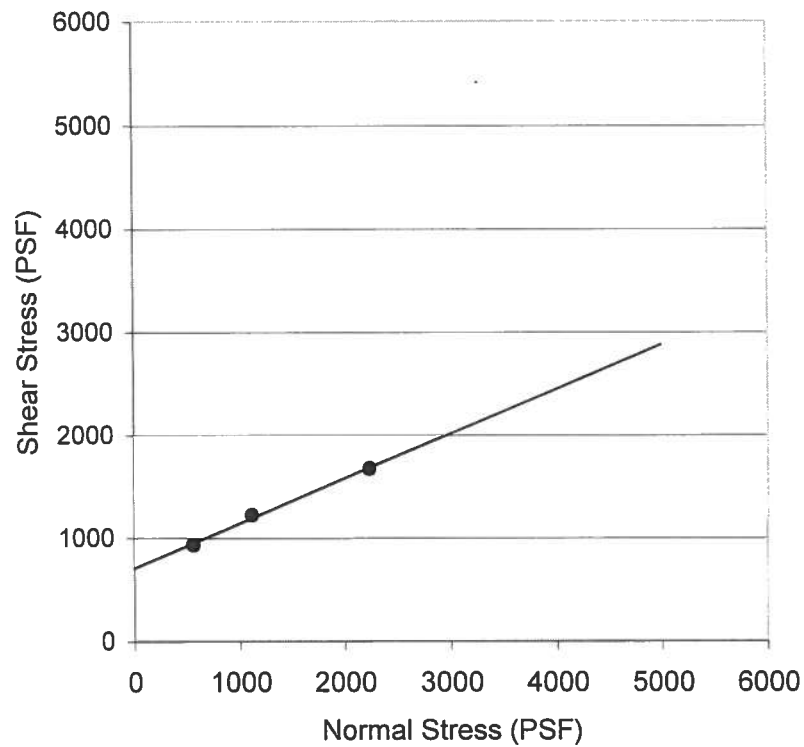
Kapolei Judiciary Complex

Hirata & Associates, Inc.

DIRECT SHEAR TEST

Plate B3.5

Direct Shear Test Results



Sample Description

Boring No.: B14 Depth (ft): 3
 Soil Description: Tan coral rubblestone
 Strength Intercept (C): 708.7 PSF (Peak Strength)
 Friction Angle (ϕ): 23.5 DEG (Peak Strength)

Remark: 5/19/03

W.O. 05-4124

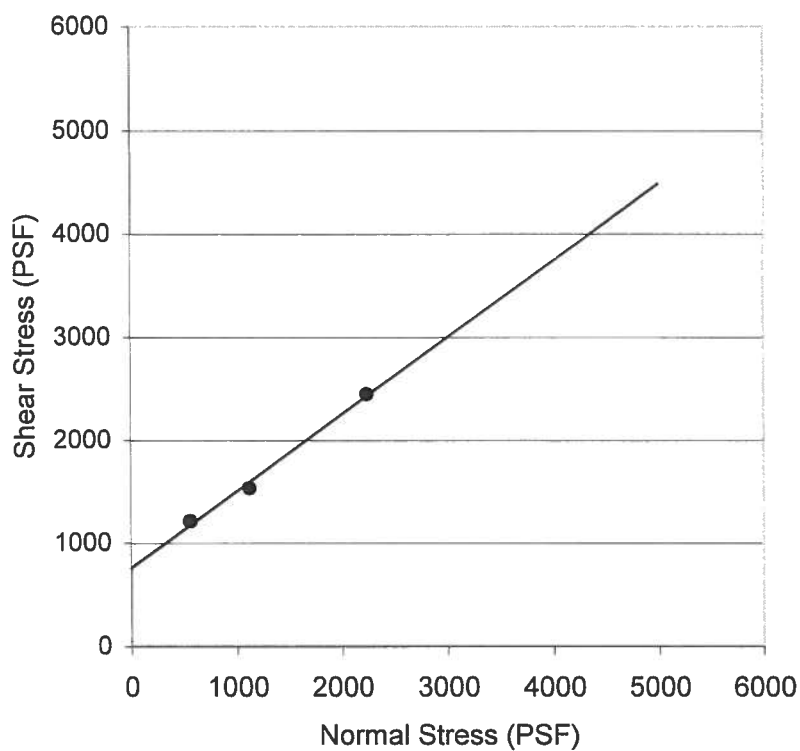
Kapolei Judiciary Complex

Hirata & Associates, Inc.

DIRECT SHEAR TEST

Plate B3.6

Direct Shear Test Results



Sample Description

Boring No.: B16 Depth (ft): 4
 Soil Description: Tan coral rubblestone
 Strength Intercept (C): 758.2 PSF (Peak Strength)
 Friction Angle (ϕ): 36.7 DEG (Peak Strength)

Remark: 12/8/05

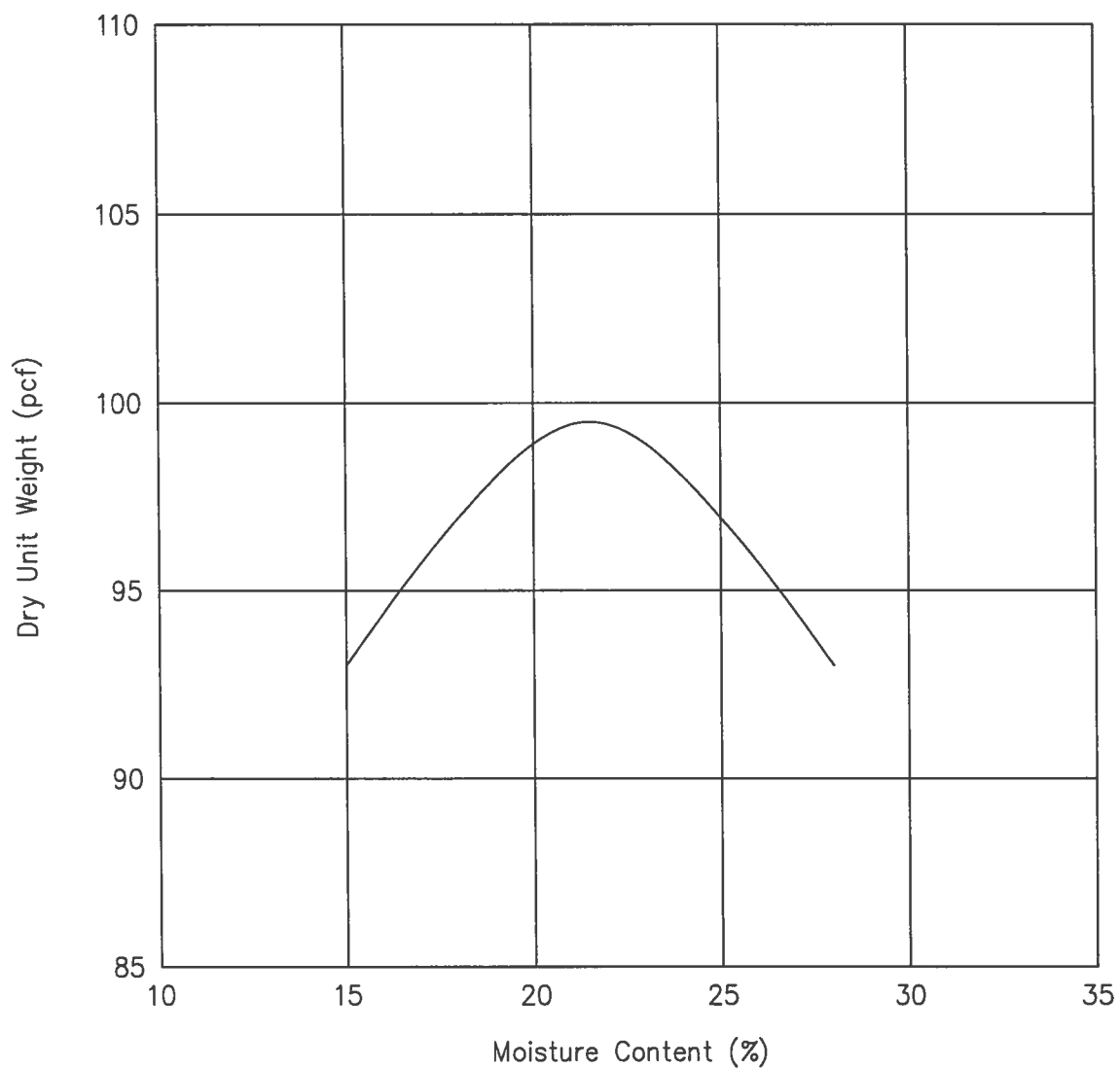
W.O. 05-4124

Kapolei Judiciary Complex

Hirata & Associates, Inc.

DIRECT SHEAR TEST

Plate B3.7



Soil Data

Location: Boring B7 from 0 to 2 feet.

Description: Grayish brown clayey silt

Test Results

Maximum Dry Density: 99.5 pcf

Optimum Moisture Content: 21.5%

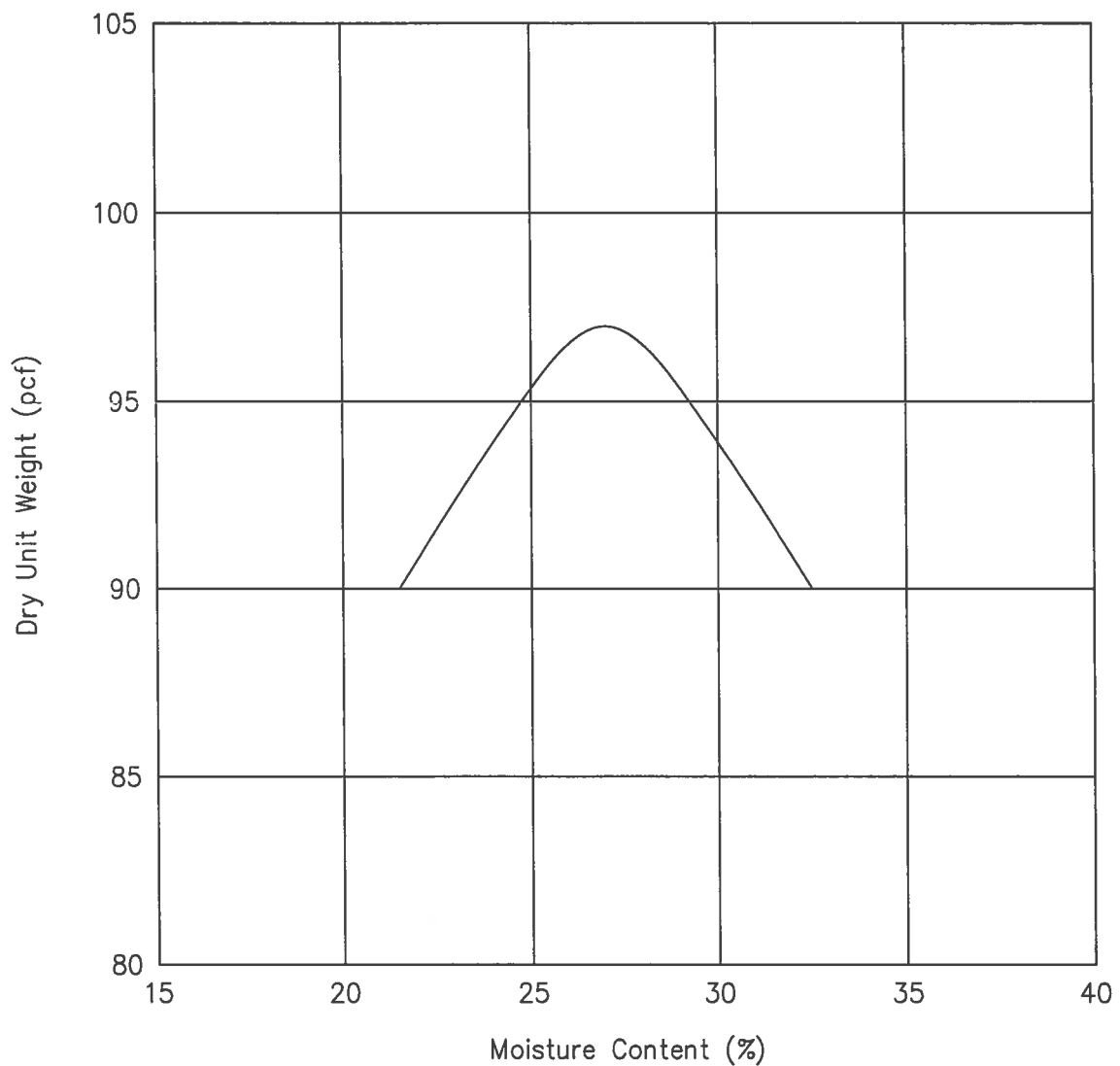
W.O. 05-4124

Kapolei Judiciary Complex

Hirata & Associates, Inc.

MODIFIED PROCTOR CURVE

Plate B4.1



Soil Data

Location: Boring B12 from 0 to 1.5 feet.

Description: Brown clayey silt

Test Results

Maximum Dry Density: 97 pcf

Optimum Moisture Content: 27%

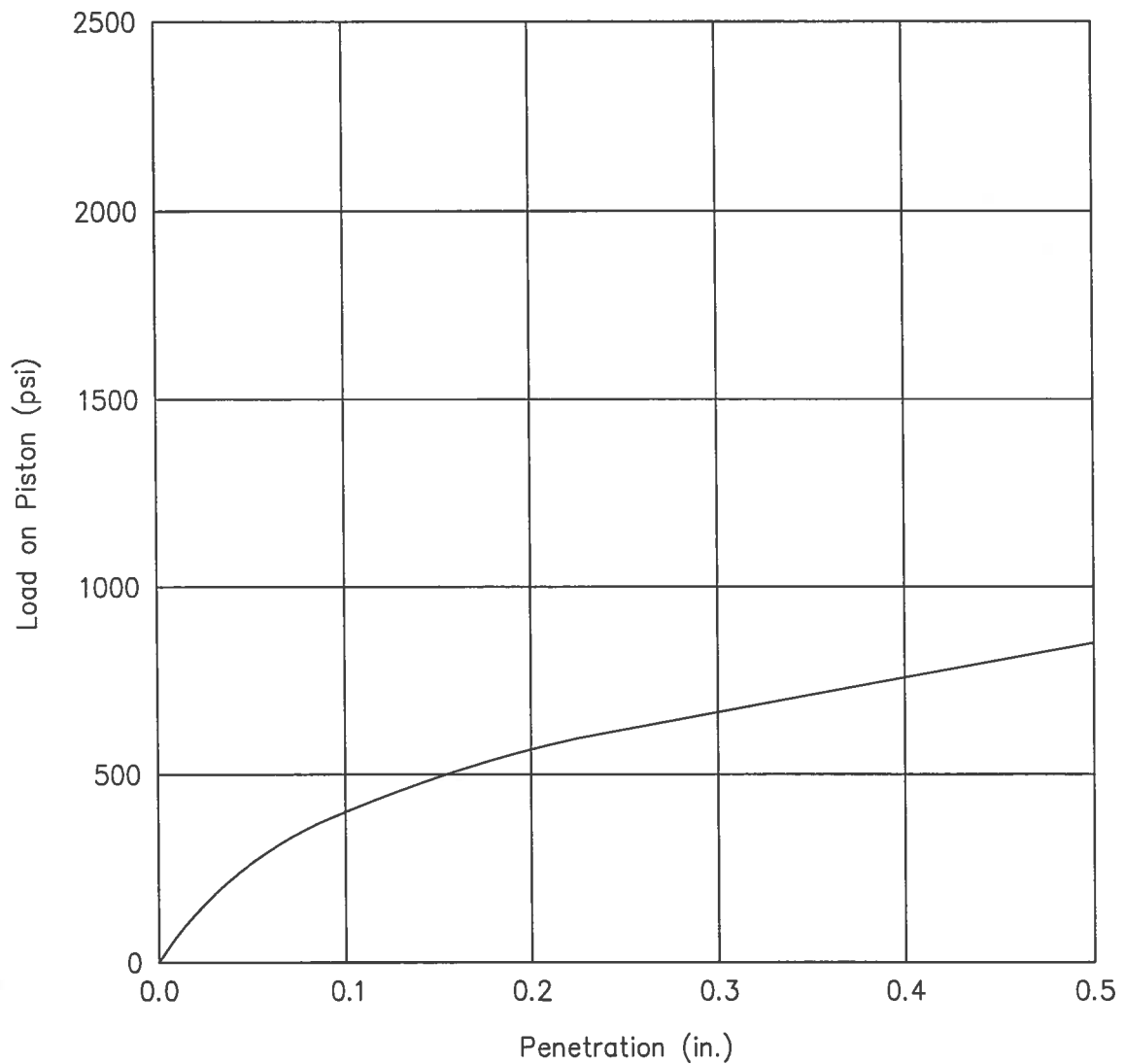
W.O. 05-4124

Kapolei Judiciary Complex

Hirata & Associates, Inc.

MODIFIED PROCTOR CURVE

Plate B4.2



Soil Data

Location: Boring B7 from 0 to 2 feet
 Description: Grayish brown clayey silt
 Sample Dry Density: 99 pcf
 Sample Moisture Content: 21%

Test Results

CBR Value: 38%
 Expansion: 0.7%

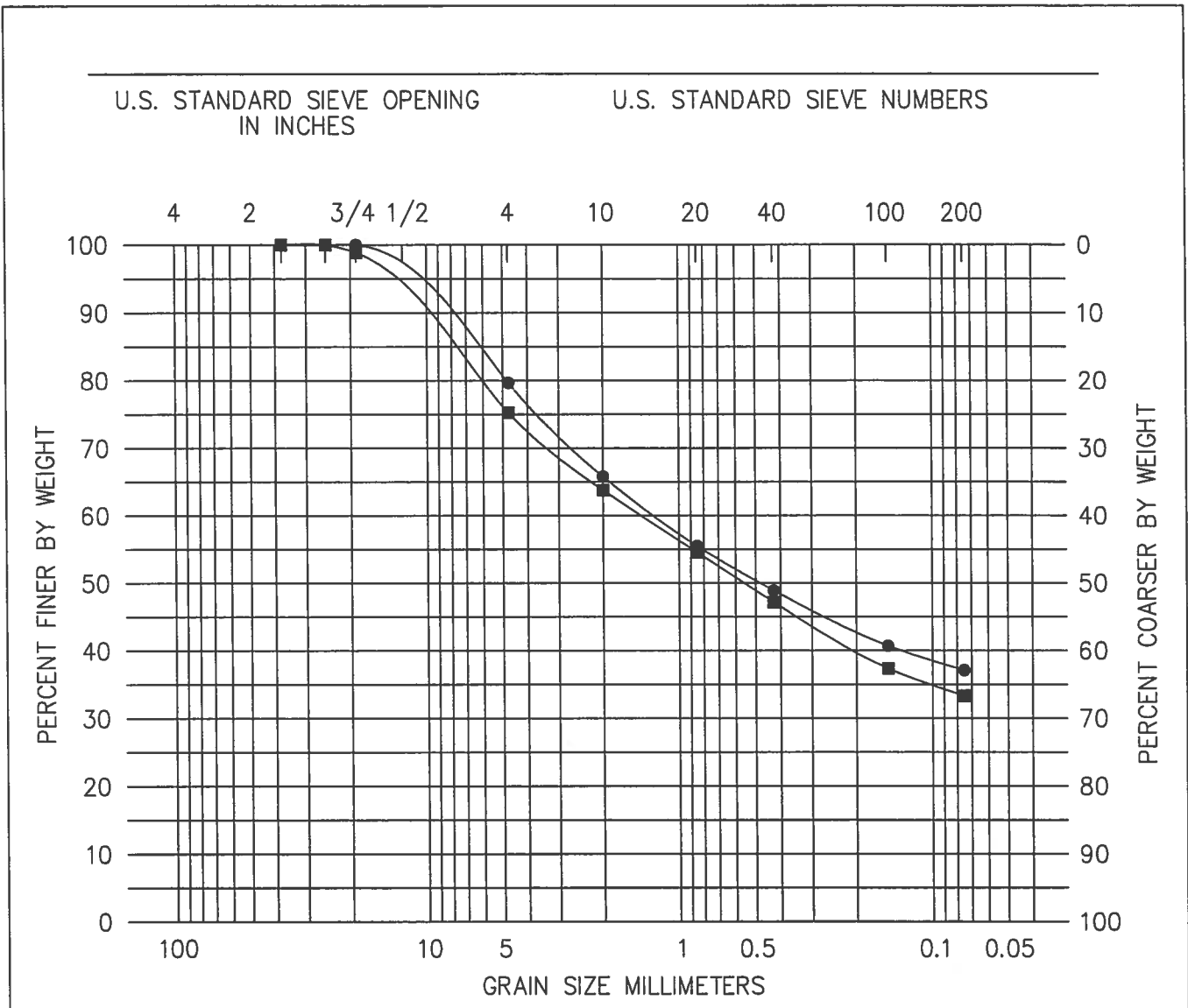
W.O. 05-4124

Kapolei Judiciary Complex

Hirata & Associates, Inc.

CBR STRESS PENETRATION CURVE

Plate B5.1

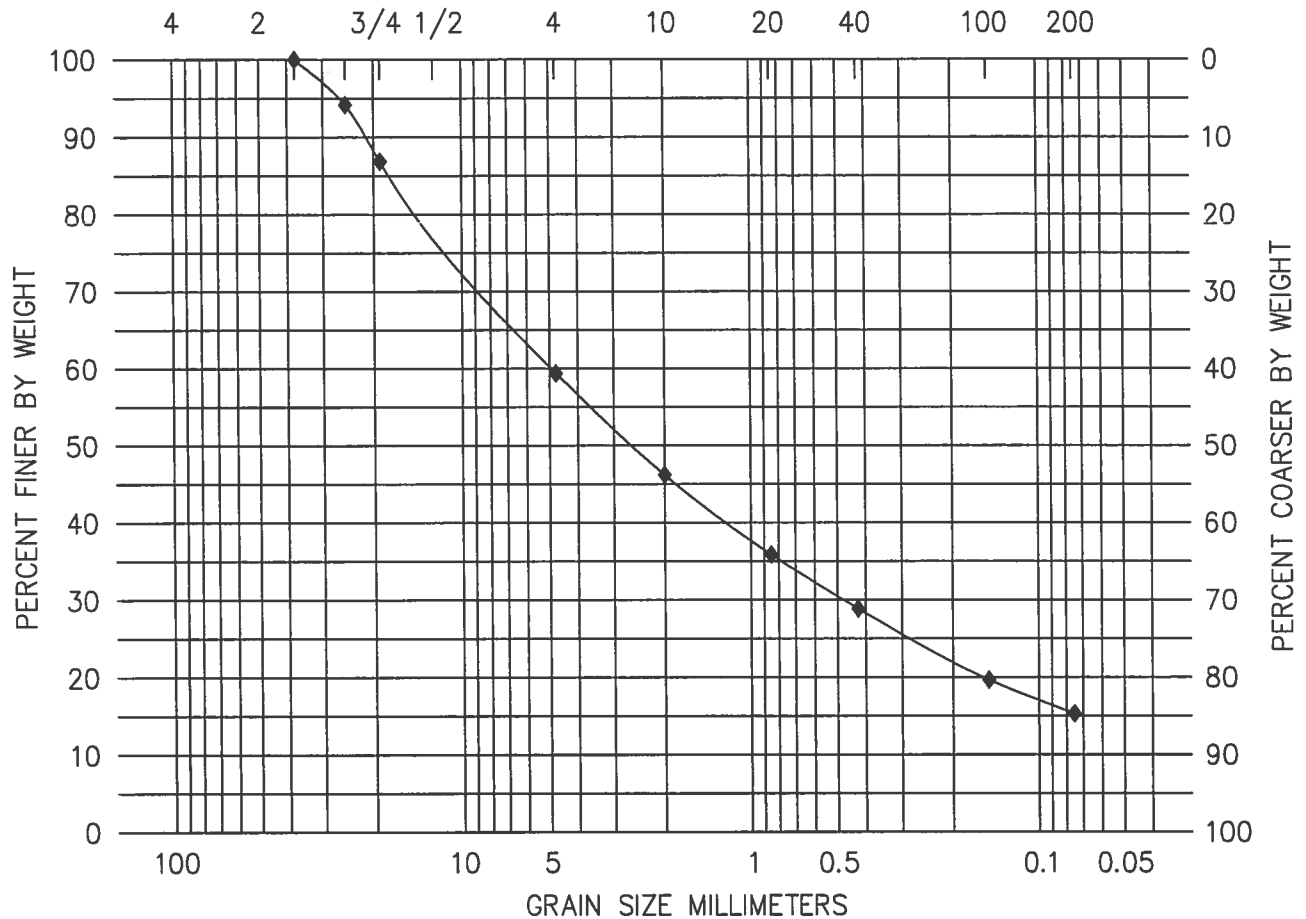


COBBLES	GRAVEL		SAND			SILT or CLAY
	Coarse	Fine	Coarse	Medium	Fine	

● Sample #1	Location: Boring B2 at 9 feet
	Description: Tan silty sand with coralline gravel (Coral Rubblestone)
■ Sample #2	Location: Boring B10 at 1 foot
	Description: Tan silty sand with coralline gravel (Coral Rubblestone)

U.S. STANDARD SIEVE OPENING
IN INCHES

U.S. STANDARD SIEVE NUMBERS

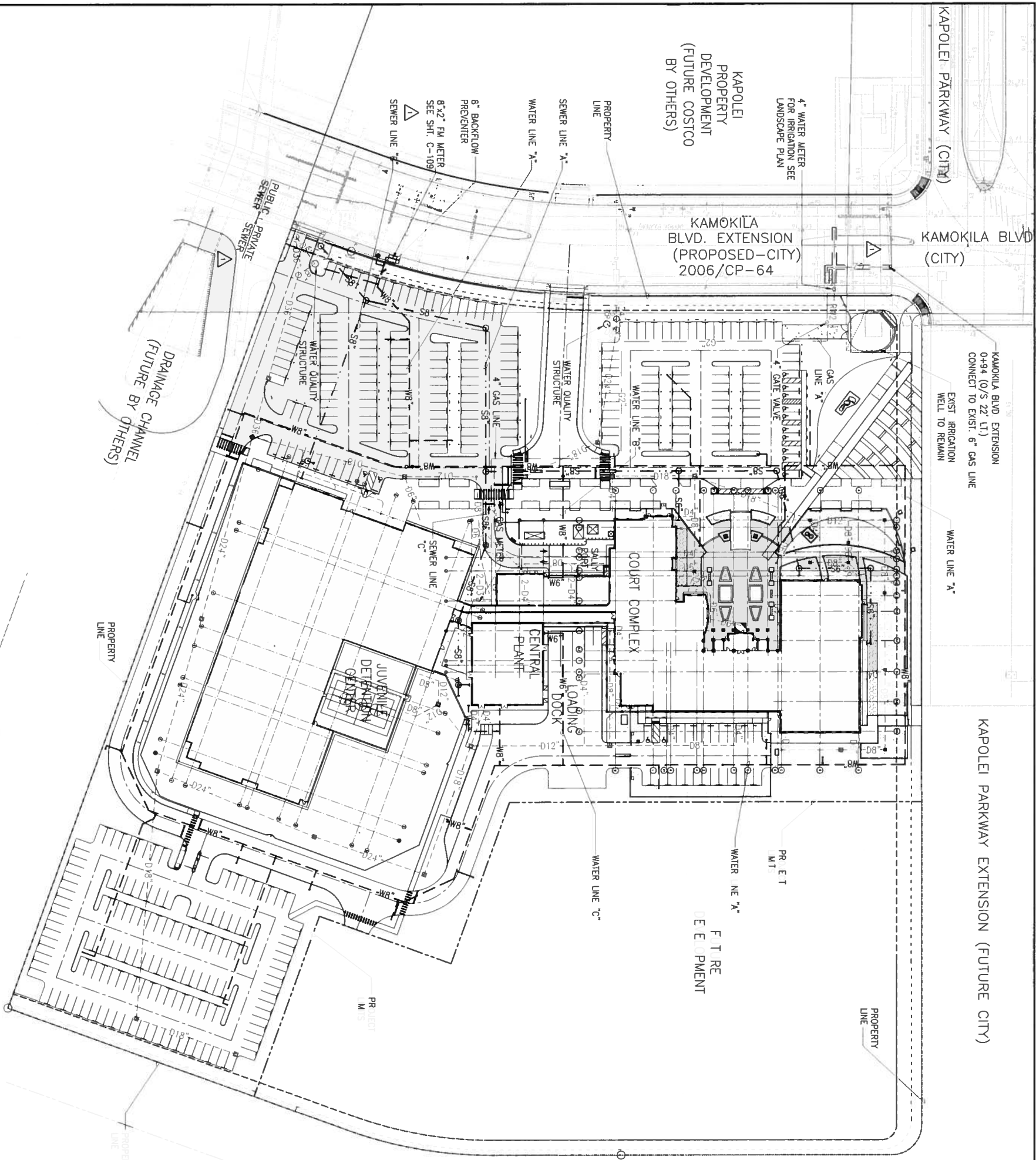


COBBLES	GRAVEL		SAND			SILT or CLAY
	Coarse	Fine	Coarse	Medium	Fine	

◆ Sample #3	Location: Boring B11 at 3 feet.
	Description: Tan silty sand with coralline gravel (Coral Rubblestone)

W.O. 05-4124	Kapolei Judiciary Complex
Hirata & Associates, Inc.	GRADATION CURVES
	Plate B6.2

Kapolei Additional Drawings



GENERAL UTILITY PLAN

SCALE: 1"=50'

TKM 9-1-016001

APPROVED:

CHIEF, CAPITAL PROJECT DIVISION, BWS

DATE

CHIEF, TRAFFIC REVIEW BRANCH, DPP

DATE

LEGEND:

- PROPERTY LINE
- WATER LINE
- DRAIN LINE
- SEWER LINE
- NON-POTABLE WATER LINE
- GAS LINE
- RAIN INLET / AREA DRAIN
- MANHOLE
- FIRE HYDRANT
- WATER METER



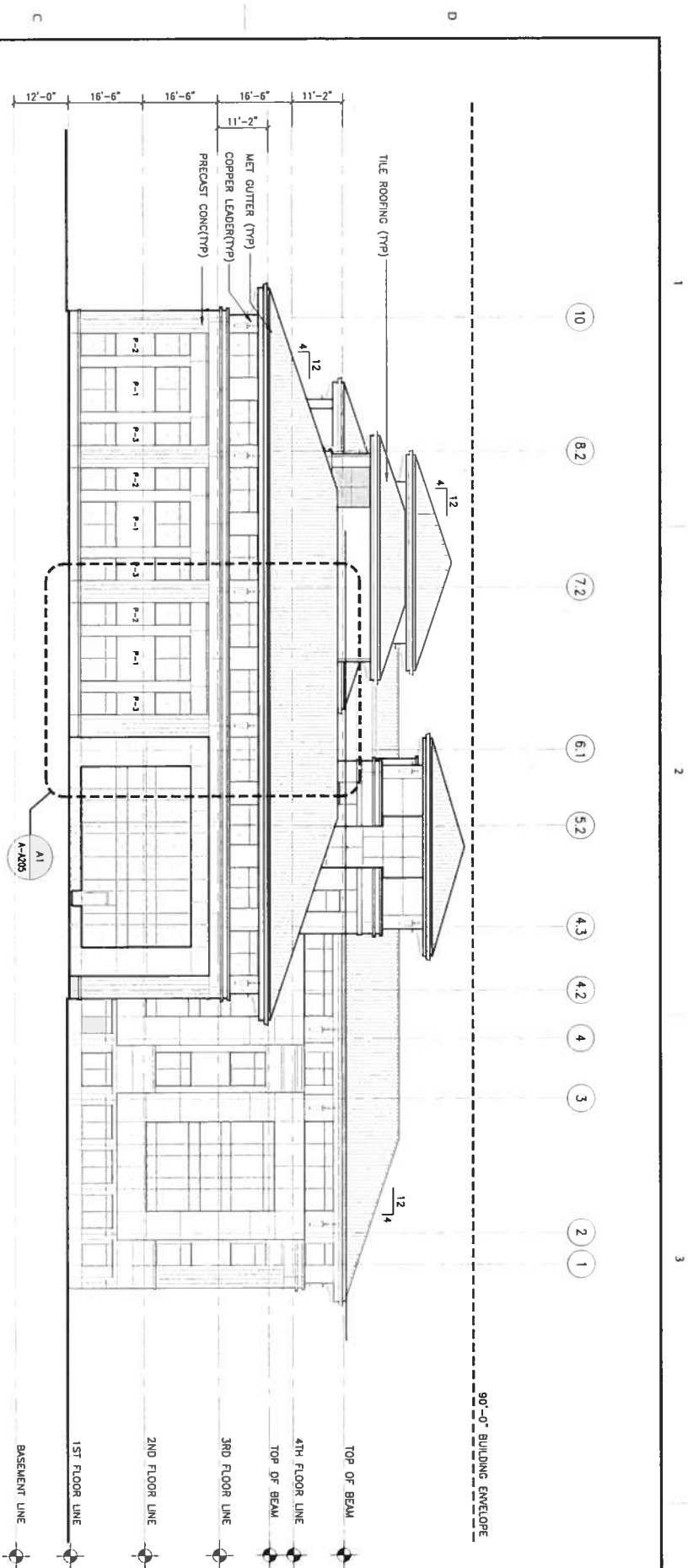
REVISION NO.	SYMBOL	DESCRIPTION	DATE	APPROVED
PCD-06	△	ADDED TRAFFIC REVIEW BRANCH APPROVAL	6/1/2008	
PCD-06	△	SOFTLINE BLOCK FOR DPP REVIEW	7/15/2007	
PCD-01	△	ADDED ROAD INTERSECTION BLOCKS AND PER CITY REVIEW COMMENTS	7/15/2007	



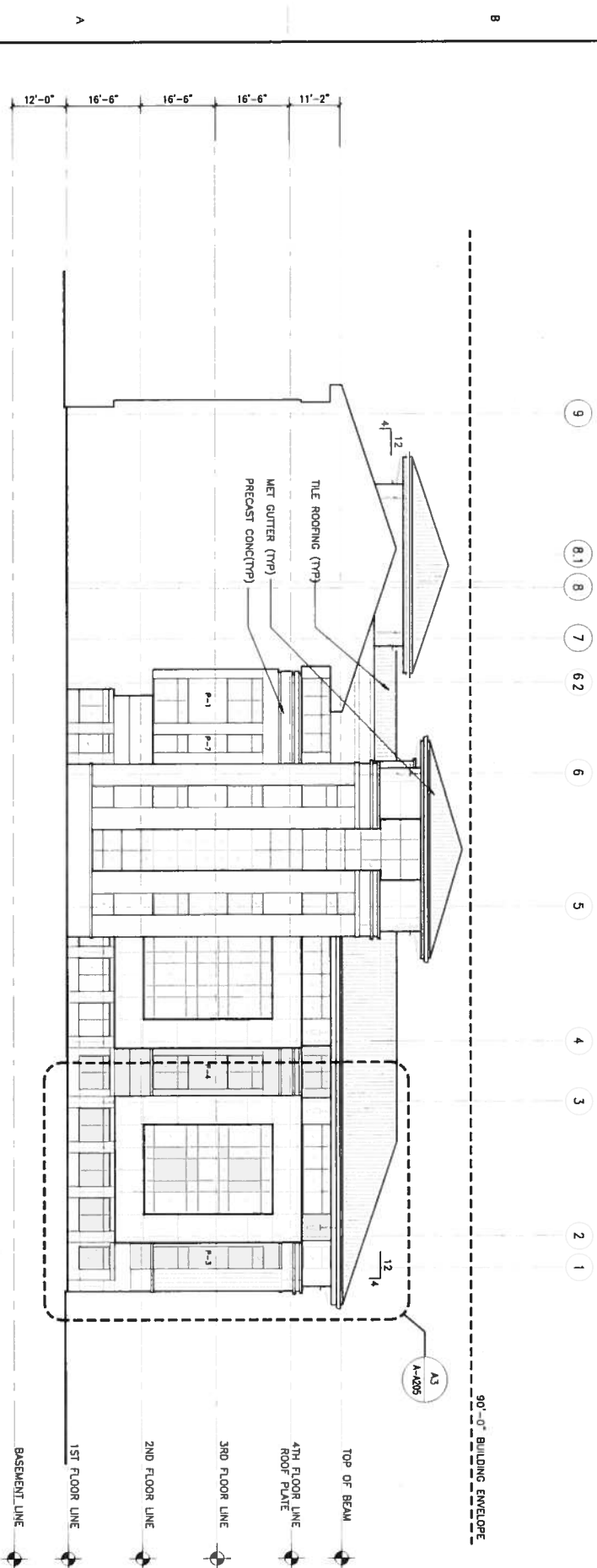
DEPT. OF ACCOUNTING & GENERAL SERVICES
DIVISION OF PUBLIC WORKS
STATE OF HAWAII
KAPOLEI JUDICIARY COMPLEX
KAPOLEI, OAHU, HAWAII

GENERAL UTILITY PLAN

MITSUNAGA & ASSOCIATES, INC.		DATE OF REV.		DRAWING NO. C-106
DESIGNED BY	CS	CHECKED BY	12-21-2003	
DRAWN BY	CS	APPROVED BY	27	
CHECKED BY	CS	DATE	NOV 2005	



C1 NORTH ELEVATION-1
SCALE: 1/16" = 1'0"



A1 NORTH ELEVATION-2
SCALE: 1/16" = 1'0"



REVISION NO.	SYN	DESCRIPTION	SUBMITTED DATE	APPROVED PUBLIC WORKS ADMINISTRATOR



This work was prepared by me or under my direct supervision and construction of this project will be under my direct supervision (Department of construction as defined in Chapter 16-115, Subchapter 1, Definitions of the Hawaii Administrative Rules, Professional Engineers, Architects, Surveyors, and Landscape Architects).

Robert M. [Signature]
signature

04/30/03
expiration date

NOTE: Contributor to charts and only dimensions at job before proceeding with work.

KAPOLEI JUDICIARY COMPLEX

**COURT COMPLEX
NORTH ELEVATION**

ARCHITECTS HAWAII, LTD.

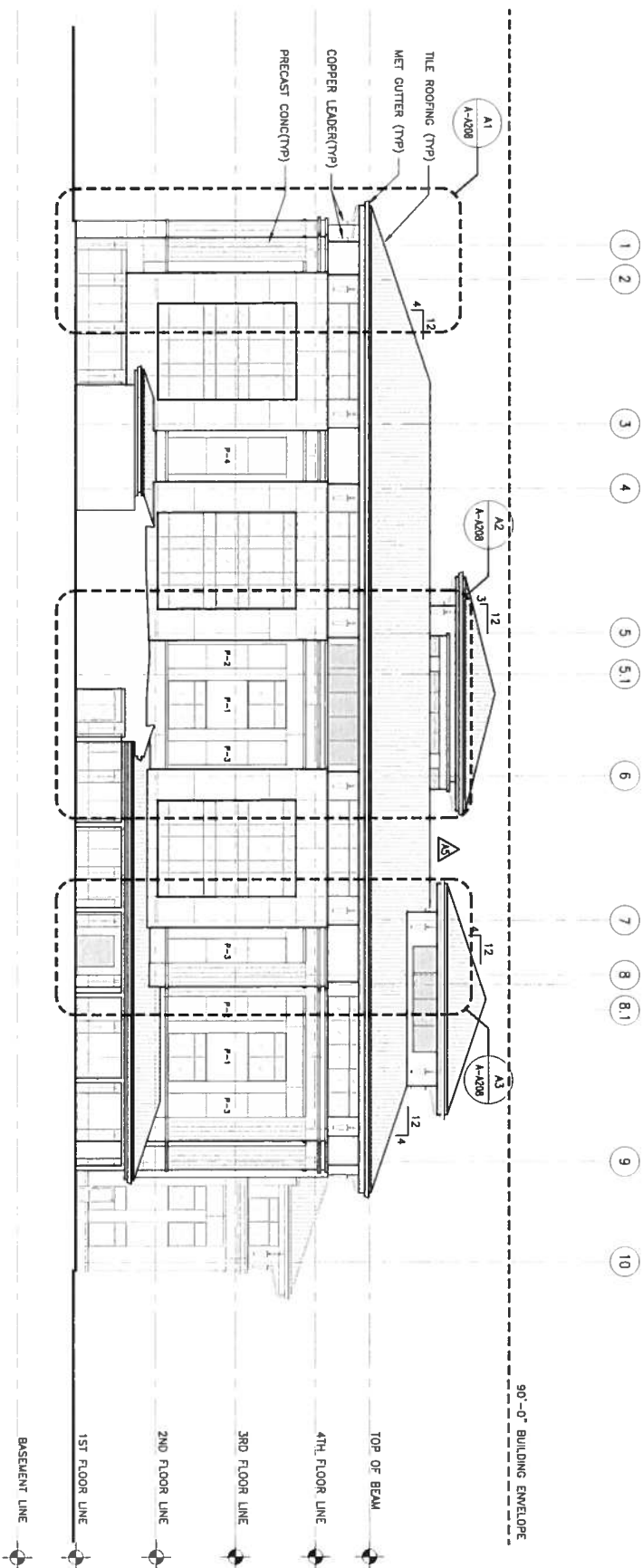
20 FT. 0-00-00 FT.

US	Approved by:
10/1/11	

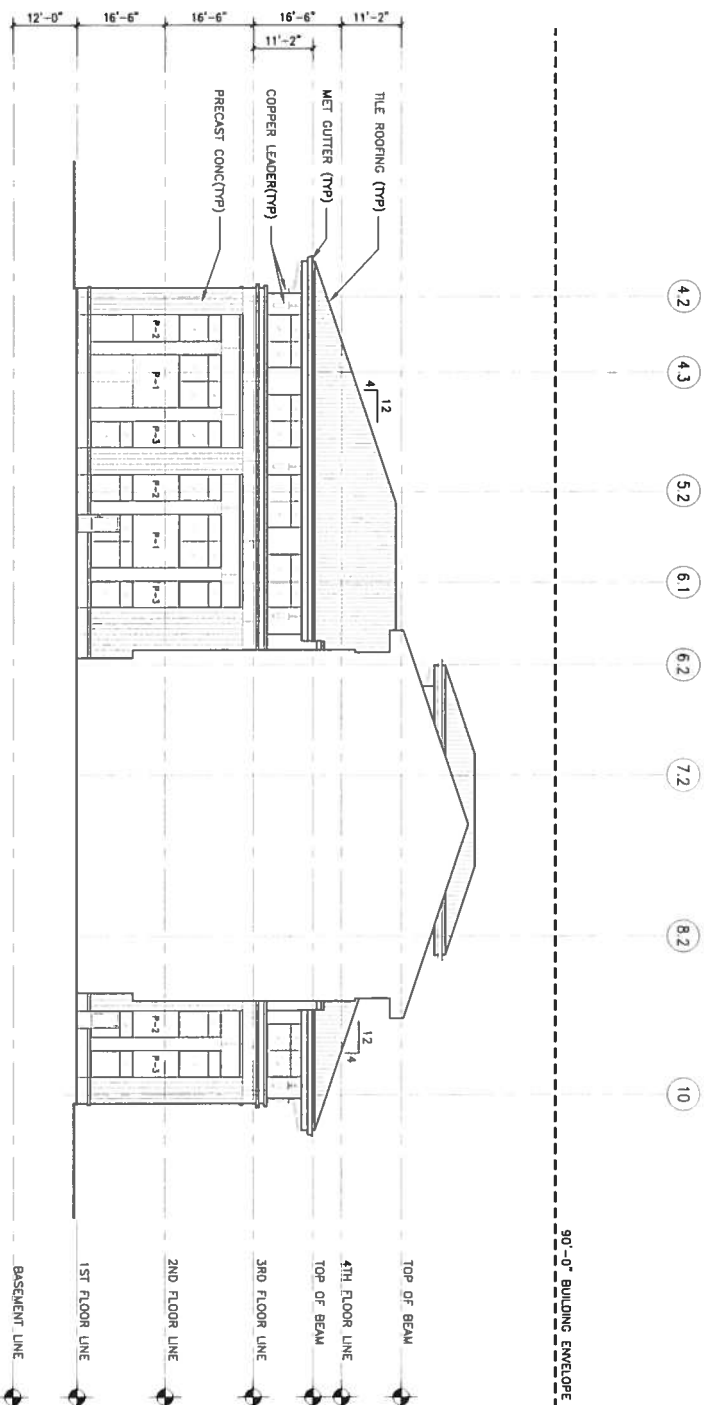
AS SHOWN

FIELD POSTED
RECORD DRAWINGS

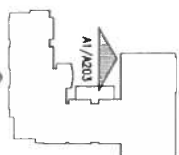
Certified by: Ed Munchy Date: 8/23/10
Unlimited Construction Services, Inc.




SOUTH ELEVATION-1
SCALE: 1/8" = 1'-0"

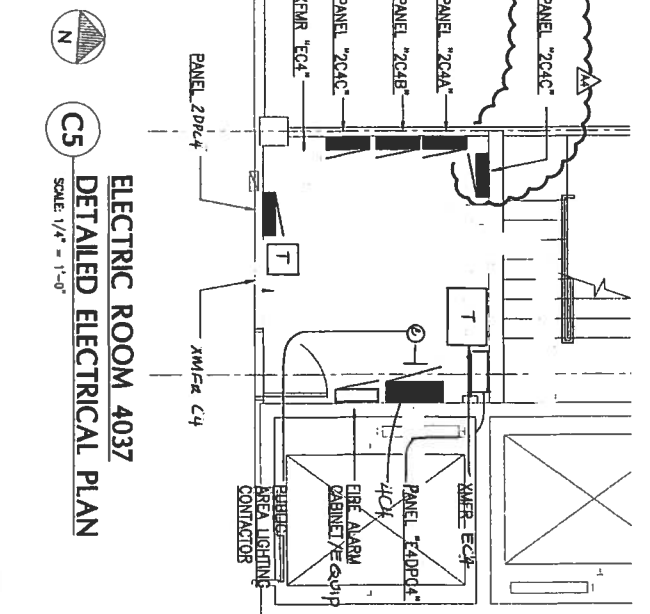


SOUTH ELEVATION-2
SCALE: 1/8" = 1'-0"

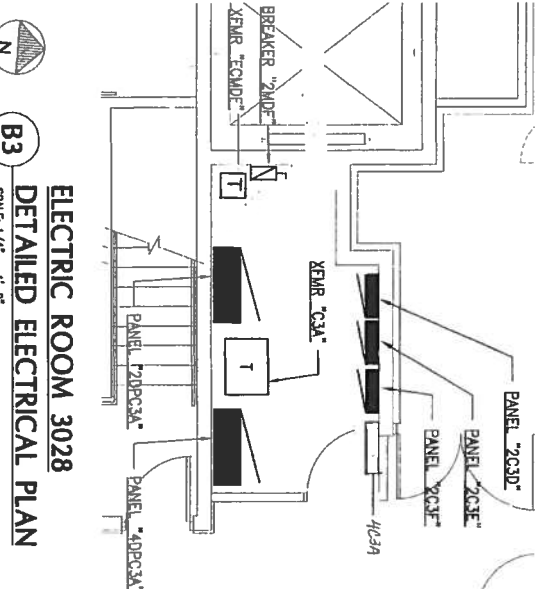


REVISION NO.	SYMBOL	DESCRIPTION	SPT. —	DATE	APPROVED: FIELD WORKS ADMINISTRATOR
ADD-05		REMOVE WINDOW TO LOWER	30 197	02/02/07	

DEPT. OF ACCOUNTING & GENERAL SERVICES DIVISION OF PUBLIC WORKS STATE OF HAWAII			
KAPOLEI JUDICIARY COMPLEX KAPOLEI OAHU, HAWAII			
COURT COMPLEX			
SOUTH ELEVATION			
ARCHITECTS HAWAII, LTD.		DRAWN BY	DATUM
DESIGNED BY	CHECKED BY	DATE	
DB	LH	12-21-1943	
APPROVED BY	SHEET	NOV 2006	
AFC	130	1021	
TOTAL	AS SHOWN		



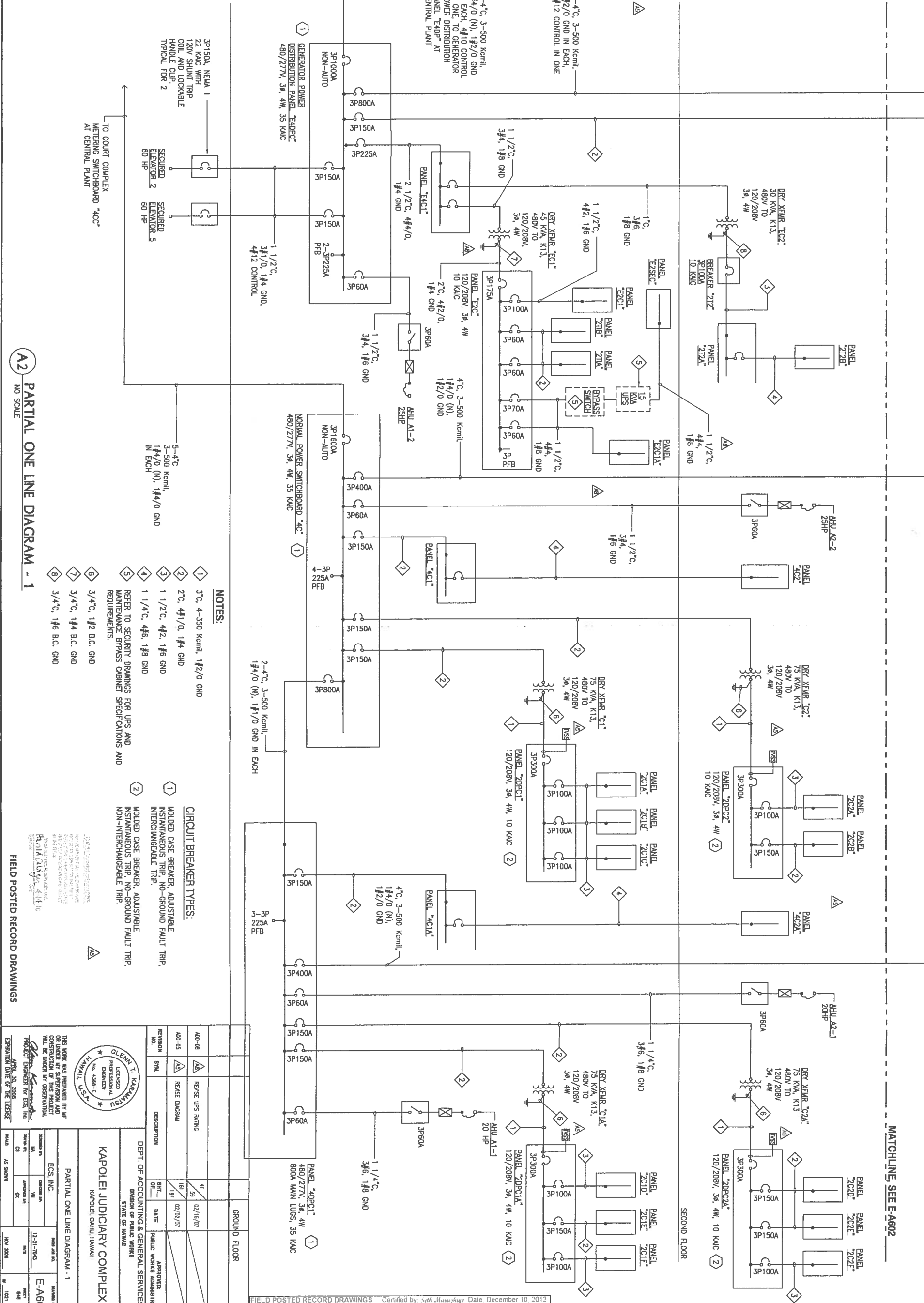
C5 **ELECTRIC ROOM 4037**
DETAILED ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"



ELECTRIC ROOM 3028
DETAILED ELECTRICAL PLAN

DEPT. OF ACCOUNTING & GENERAL SERVICES DIVISION OF PUBLIC WORKS STATE OF HAWAII	
KAPOLEI JUDICIARY COMPLEX KAPOLEI, OAHU, HAWAII	
ELECTRIC ROOM DETAIL PLANS	
PROJECT ENGINEER FOR E&S, INC. APRIL 30, 2008 EXPIRATION DATE OF THE LICENSE	THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION FOR THE CONSTRUCTION OF THIS PROJECT WILL BE UNDER MY OBSERVATION.
E&S, INC. LICENSE NO. 12-21-7043 DATE AND REG.	E-A-401 SHEET
DESIGNED BY: MA DRAWN BY: JENNIFER HUI CHECKED BY: DK SCALE: AS SHOWN	MOY 2008 OF _____ SHEETS

FIELD POSTED RECORD DRAWINGS Certified by Seth Marushige Date December 10, 2012
Unlimited Construction Services, Inc.



A2 PARTIAL ONE LINE DIAGRAM - 1
NO SCALE

FIELD POSTED RECORD DRAWINGS

THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION AND CONSTRUCTION OF THIS PROJECT WILL BE UNDER MY OBSERVATION.

Glenn T. Karamitsu
Professional Engineer
No. 4380
HAWAII, U.S.A.

APPROVED BY:
DATE: 02/16/07
DATE: 02/02/07

PROJECT ENGINEER FOR ECOS, INC.
Huihui Chua, P.E.

EXPIRATION DATE OF THE LICENSE:
APRIL 30, 2008

DEPT. OF ACCOUNTING & GENERAL SERVICES
DIVISION OF PUBLIC WORKS
STATE OF HAWAII

KAPOLEI JUDICIARY COMPLEX
KAPOLEI, OAHU, HAWAII

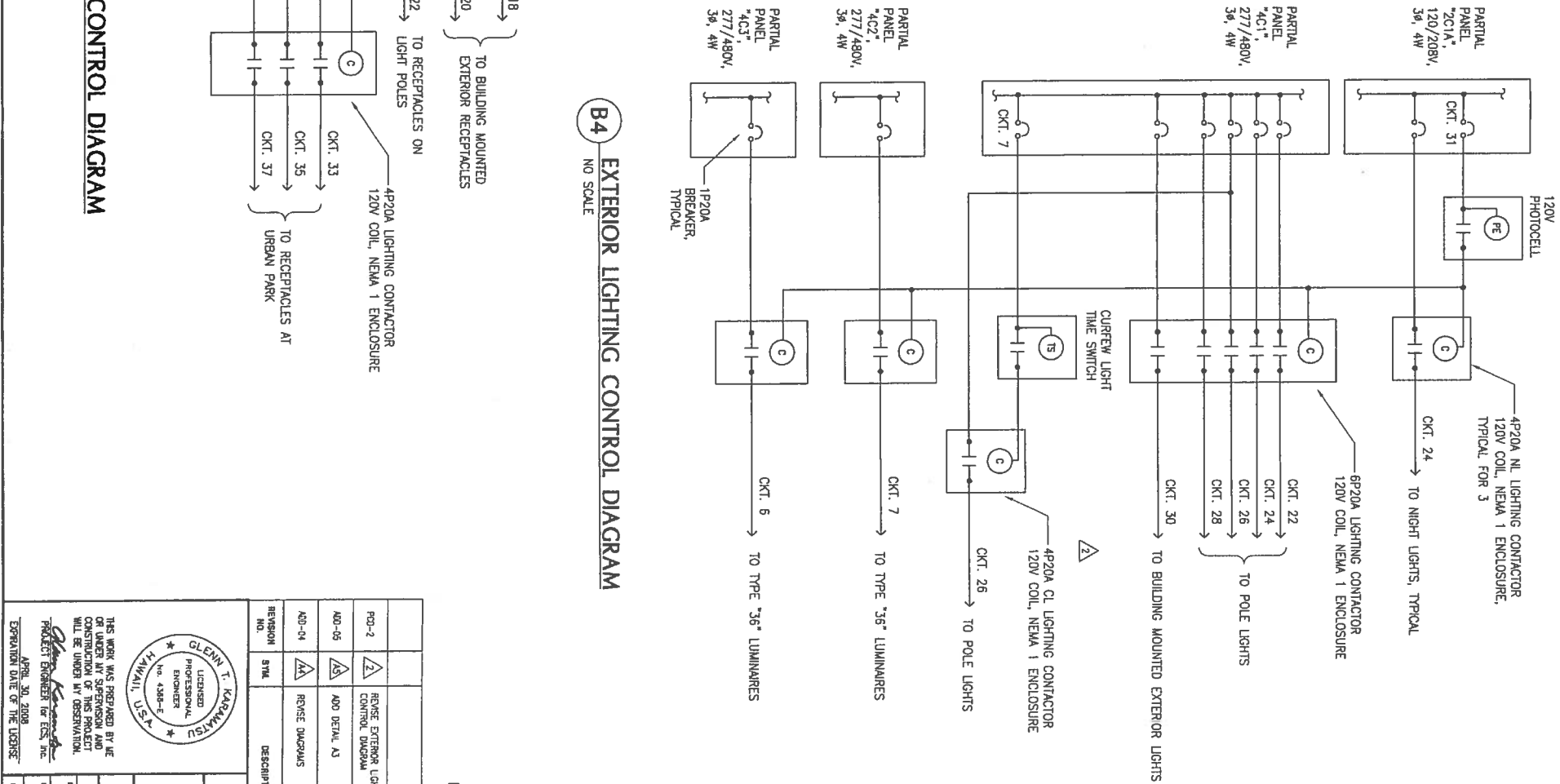
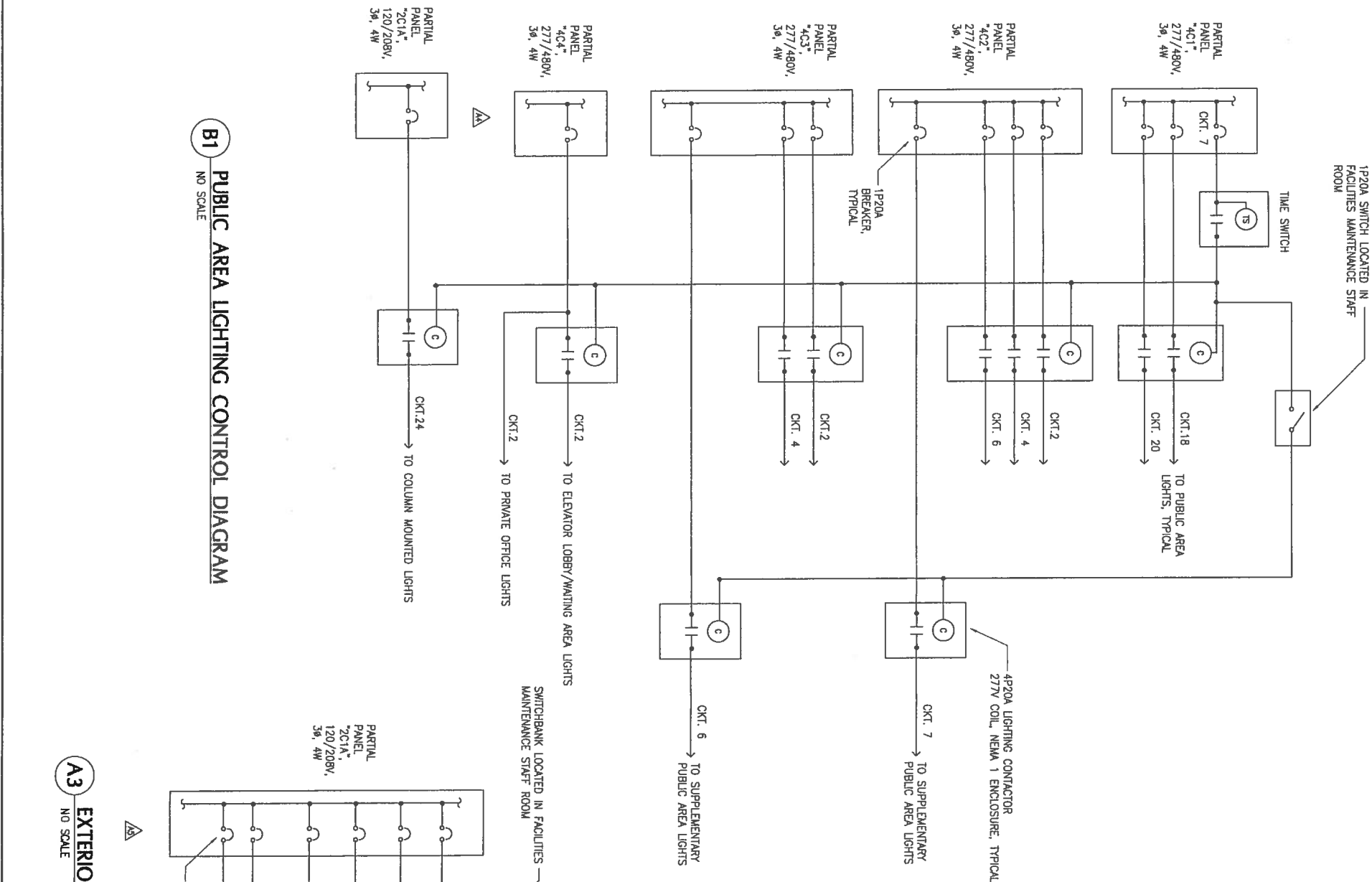
PARTIAL ONE LINE DIAGRAM - 1

ECOS, INC.
12-21-2003
E-A601

APPROVED BY:
DATE: 02/16/07
DATE: 02/02/07

PROJECT ENGINEER FOR ECOS, INC.
Huihui Chua, P.E.

EXPIRATION DATE OF THE LICENSE:
APRIL 30, 2008



FIELD POSTED RECORD DRAWINGS			
NO.	REVISION	DATE	APPROVED
POD-2	REVISE EXTERIOR LIGHTING CONTROL DIAGRAM	10/26/07	
ADD-05	ADD DETAIL A3	02/02/07	
ADD-04	REVISE DIAGRAMS	07/26/07	
REVISION NO.	SYMBOL	DESCRIPTION	DATE
1	Δ	REVISE EXTERIOR LIGHTING CONTROL DIAGRAM	10/26/07
2	Δ	ADD DETAIL A3	02/02/07
3	Δ	REVISE DIAGRAMS	07/26/07
DEPT. OF ACCOUNTING & GENERAL SERVICES DIVISION OF PUBLIC WORKS STATE OF HAWAII			
KAPOLEI JUDICIARY COMPLEX KAPOLEI, OAHU, HAWAII			
LIGHTING CONTROL DIAGRAMS			
DESIGNED BY	ECS, INC.	DRAWN BY	12-21-7043
CHECKED BY	VA	DATE	04/30/2008
APPROVED BY	CS	DATE	05/11/2011
SCALE	AS SHOWN	DATE	05/11/2011

THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION AND CONSTRUCTION OF THIS PROJECT WILL BE UNDER MY OBSERVATION.

Glenn T. Karamatsu
PROJECT ENGINEER FOR ECS, INC.
APRIL 30, 2008
EXPIRATION DATE OF THE LICENSE

GLENN T. KARAMATSU
LICENSED PROFESSIONAL ENGINEER
NO. 4388-E
HAWAII, U.S.A.

FIELD POSTED RECORD DRAWINGS

Certified by *John H. H. H. H.* Date December 10, 2012
Unlimited Construction Services, Inc.

[illegible]

PANEL "E4C3"										277/480 VOLTS, 3 PHASE, 4 WIRE		MIN. AIC: 14,000 MOUNTING: SURFACE	
WIRE SIZE (AWG)		225 AMP MAIN LUGS		CONNECTED LOAD (KVA)				CT BKR		WIRE SIZE (AWG)			
WIRE NO	CT	USE	POLE/AMP	PHASE A	PHASE B	PHASE C	AMP/POLE	USE	CT	WIRE NO			
1	1 PERB	1	1	1.5	1.5	2	20	LIGHTS-3RD FLOOR	2	10			
3	3 PERB	1	1				20	LIGHTS-4TH FLOOR	4	10			
5	5 PERB	1	1				20	SPACE	6	8			
7	7 PERB	1	1			1.5			8	8			
9	9 PERB	1	1						10	8			
11	11 PERB	1	1						12	8			
6	13 PANEL "213A"	3	3	8.0					14	16			
6	15 PANEL "213A"	6	6		8.0				16	18			
6	17	6	6			8.0			18	20			
4	21 PANEL "2AMD"	3	3	11.7					20	22			
4	23	80	80		11.7				22	24			
CONNECTED LOAD/PHASE				21.1	21.1	21.2							
TOTAL CONNECTED LOAD				63.6 KVA									
DEMAND FACTOR				0.8									
TOTAL DEMAND LOAD				50.9 KVA=			61 AMPS						

227/480 VOLTS, 3 PHASE, 4 WIRE										MIN. A/C: 14,000	
225 AMP MAIN LUGS										MOUNTING: SURFACE	
PANEL "4C1" II										WIRE SIZE (AWG)	
CKT NO	USE	CKT BKFR	CONNECTED LOAD (KVA)	PHASE A	PHASE B	PHASE C	CKT BKFR	USE	WIRE SIZE (AWG)		
		POLE/AMP					AMP/PAE		NO		
12	1						20	1	LTS-BASEMENT	2	12
12	3	EF A1-2	0.8 2.1				20	1	LTS-RCCG/HOOD/OND	4	12
12	5			0.8	2.0		20	1	LTS-FAC MAINT	6	12
12	7	TIME SWITCH					20	1	LTS-SHIFT	8	12
12	9	SPACE	1	20			20	1	LTS-PRV LOBBY	10	12
12	11	SPACE	1	20	1.5	2.0	20	1	LTS-DOCS CLERKS	12	12
10	13	VENDOR-WATER HTR	1	30			20	1	LTS-DOCS CLERKS	14	12
10	15	VENDOR-WATER HTR	1	30	6.6	1.8	20	1	LTS-ACCTS CLERKS	16	12
17	PRB						20	1	LTS-CLERK COUNTER	18	12
19	PRB						20	1	LTS-LOBBY/STAR	20	12
21	PRB			—	2.1		20	1	LTS-LOBBY/STAR	22	10
23	PRB						20	1	LTS-LOBBY/STAR	22	10
4	25	PRB			—	2.0	20	1	LTS-JUDGES PKG	24	10
4	25	PRB					20	1	LTS-STAFF PKG	24	10
4	25	PRB					20	1	LTS-STAFF PKG	24	10
4	25	PRB					20	1	LTS-PUBLIC PKG	26	10
4	27	PANEL "4C2"	11.0 2.1				20	1	LTS-PUBLIC PKG	26	10
4	29		60	11.0 2.0			20	1	LTS-BLDG EXT	30	10
CONNECTED LOAD/PHASE			29.0	29.9	23.1						
TOTAL CONNECTED LOAD			85.0	KVA							
DEMAND FACTOR			1.0								
TOTAL DEMAND LOAD			82.0	KVA=	98	MAWS					

TC- DENOTES TIME CLOCK
CONTROLLED LIGHTING CIRCUIT

PANEL "4C1A"										277/480 VOLTS, 3 PHASE, 4 WIRE 100 AMP MAIN LUGS		NIN AGC - 14,000 MOUNTING: SURFACE	
WIRE SIZE (AWG)		USE	CKT BKR	CONNECTED LOAD (KVA)			CMT BKR	USE	CMT	SIZE (AWG)			
NO	NO			POLE/AMP	PHASE A	PHASE B					PHASE C	AMP/POLE	NO
12	1	3	2.1	2.1		20	1	LTS-JUDGE/CLERKS	4	12			
12	5	15		2.1	2.2	20	1	LTS-CLERKS	6	12			
12	5	1	2.0		2.1	2.2	20	1	LTS-CLERKS	8	12		
-	7 PFB	1	-	1.9		20	1	LTS-COURTROOM	10	12			
-	9 PFB	1	-		1.6	20	1	LTS-HOLDING CELLS	12	12			
-	11 PFB	1	-			20	1	LTS-COURTROOM	14	12			
6	13	3	8.0	2.0		20	1	SPARE	16	12			
6	15			8.0	2.0	20	1	SPARE	18	12			
6	17		60		8.0	2.0	20	1	SPARE	18	12		
CONNECTED LOAD/PHASE				16.2	16.2	15.9							
TOTAL CONNECTED LOAD				48.5	KVA								
DEMAND FACTOR				1.0									
TOTAL DEMAND LOAD				48.3	KVA=	58	AMPS						

PANEL "4C2"										277/480 VOLTS, 3 PHASE, 4 WIRE 100 AMP MAIN LUGS		MIN AINT: 14,000 MOUNTING: SURFACE		
WIRE SIZE (AWG)		CIRCUIT		USE		CIRCUIT BREAKER		CONNECTED LOAD (KVA)		CIRCUIT BREAKER		WIRE SIZE (AWG)		
NO	SIZE	NO	DESCRIPTION	USE	PHASE A	PHASE B	PHASE C	AMPS	POLE	NO	DESCRIPTION	USE	NO	SIZE
12	3	1	FCU A2-2	3	3.0	1.7		20	1	1	LTS-LOBBY	2	12	
12	3	2						20	1	2	LTS-LOBBY	4	12	
12	5	3	LTS-WINDOWS	20		3.0	1.7	20	1	3	LTS-WAITING/HALL	6	12	
12	5	4		20				20	1	4	LTS-PRIV CORRIDOR	8	12	
9		5	SPACE	1	2.0	2.2		20	1	5	LTS-COURT 2071	10	12	
11		6	SPACE	1	20			20	1	6	LTS-COURT 2080	10	12	
13	PEB	7						20	1	7	LTS-COURT 2087	14	12	
16	PEB	8			2.3			20	1	8	LTS-COURT 2058	16	12	
17	PEB	9				2.2		20	1	9	SPACE	18		
		CONNECTED LOAD/PHASE		11.0		10.8		10.8						
		TOTAL CONNECTED LOAD		32.6		KVA								
		DEMAND FACTOR		1.0										
		TOTAL DEMAND LOAD		32.6		KVA= 39 AMPS								
T-C - DENOTES TIME CLOCK CONTROLLED LIGHTING CIRCUIT														

PANEL "4C2A"														277/480 VOLTS, 3 PHASE, 4 WIRE 100 AMP MAIN LUGS				MIN AIG: 14,000 MOUNTING: SURFACE			
(AUG)		SIZE		CMT		CONNECTED LOAD (KVA)						CMT		(AUG)							
NO	NO	USE	POLE	AMP	PHASE A	PHASE B	PHASE C	AMP	POLE	USE	NO	NO									
12	1	1	3		1.0	2.2			20	1	LTS-COURT 2043	2	12								
12	3	EF A2-1							20	1	LTS-COURT 2052	4	12								
12	5		15				1.0	2.0	20	1	LTS-ADMIN STAFF	6	12								
7		SPARE			2.0	2.2			20	1	LTS-ADMIN STAFF	8	12								
9		SPARE	1	20			2.0	2.0	20	1	LTS-BRACHES/PROG	10	12								
11		SPARE	1	20			2.0	2.1	20	1	LTS-BRACHES/PROG	12	12								
13		PER	1		2.0				20	1	LTS-JCS	14	12								
15		PER	1			1.9			20	1	LTS-JCS	16	12								
17		PER	1				2.0		20	1	SPARE	18									
CONNECTED LOAD/PHASE					9.4	9.1	9.1														
TOTAL CONNECTED LOAD					27.6	KVA															
DEMAND FACTOR					1.0																
TOTAL DEMAND LOAD					27.6	KVA=	32	AMPS													

PANEL "4C3"										277/480 VOLTS, 3 PHASE, 4 WIRE 100 AMP MAIN LUGS		MIN AIC: 14,000 MOUNTING: SURFACE	
WIRE SIZE (AWG)		Circuit Breaker		Connected Load (KVA)		Circuit Breaker		WIRE SIZE (AWG)		MIN AIC: 14,000 MOUNTING: SURFACE			
NO	USE	POLE/AMP	PHASE A	PHASE B	PHASE C	AMP/POLE	USE	NO	WIRE SIZE (AWG)				
12 1		3	3.0 1.7			20 1	L1S-LOBBY	4	12				
12 3	FCU A3-2	20		3.0 1.7		20 1	L1S-LOBBY	4	12				
12 5		20			3.0 1.7	20 1	L1S-WINDOWS	6	12				
7	PEB	1	2.0			20 1	L1S-PKIN CORRIDOR	8	12				
9	PEB	1		2.2		20 1	L1S-COURT 3064	10	12				
11	PEB	1			2.2	20 1	L1S-COURT 3073	12	12				
13	PEB	1	2.2			20 1	L1S-COURT 3081	14	12				
15	PEB	1		2.2		20 1	L1S-COURT 3051	16	12				
17	PEB	1				20 1	SPACE	18	12				
19	PEB	1	2.0			20 1	SPACE	20	12				
21	PEB	1		2.0		20 1	SPACE	22	12				
23	PEB	1			2.0	20 1	SPACE	24	12				
CONNECTED LOAD/PHASE			10.9	11.1	10.9								
TOTAL CONNECTED LOAD			32.9	KVA									
DEMAND FACTOR			1.0										
TOTAL DEMAND LOAD			32.9	KVA=	39 AMPS								

"C" DENOTES TIME CLOCK
CONTROLLED LIGHTING CIRCUIT

PANEL "4C3A"												277/480 VOLTS, 3 PHASE, 4 WIRE 100 AMP MAIN LUGS		MIN AIC: 14,000 MOUNTING: SURFACE	
WIRE SIZE (AWG)		CMT BKRR		CONNECTED LOAD (KVA)		CMT BKRR		USE		CMT BKRR		WIRE SIZE (AWG)			
NO	USE	POLE/AMP	PHASE A	PHASE B	PHASE C	AMP/POLE	NO	USE	NO	USE	NO	USE			
12	3	3	2.0	1.9		20	1	LTS-COURT 30.35	4	12	2	12			
12	3	3			2.0	20	1	LTS-COURT 30.45	5	12	2	12			
12	5	15			2.0	20	1	LTS-CLERKS	6	12	2	12			
7	SPACE	1	2.0	1.8		20	1	LTS-CLERKS	8	12	2	12			
9	SPACE	1	2.0		2.0	20	1	LTS-CLERKS	10	12	2	12			
11	SPACE	1	2.0		2.0	20	1	LTS-CLIENT SVCS	12	12	2	12			
13	PFB	1	1.8			20	1	LTS-CLIENT SVCS	14	12	2	12			
15	PFB	1			2.0	20	1	LTS-MULTI-PURPOSE	16	12	2	12			
17	PFB	1				20	1	LTS-BALUF	18	12	2	12			
CONNECTED LOAD/PHASE			10.5	10.8		10.6									
TOTAL CONNECTED LOAD			31.9	KVA											
DEMAND FACTOR			1.0												
TOTAL DEMAND LOAD			31.9	KVA=	38	AMPS=									

PANEL "4C4"										277/480 VOLTS, 3 PHASE, 4 WIRE 100 AMP MAIN LUGS		MIN. A/C: 14,000 MOUNTING: SURFACE	
SIZE WIRE NO	CT	USE	POLE/AMP	CONNECTED LOAD (KVA)			CT BKTR	USE	NO	SIZE WIRE NO			
12/1	1	EF A4-1	3	1.2/2.0	1.2/2.0		20/1	LTS-LOBBY/MAINING	2	12			
12/5	1	EF A4-1	15		1.2/2.0		20/1	LTS-CLERKS/CONF	4	12			
12/7	1	EF A4-2	3	0.5/1.9		1.2/2.0	20/1	LTS-PRIV LOBBY	6	12			
12/11	1	EF A4-2	15		0.5/1.9		20/1	LTS-CLERKS	8	12			
12/12	1		3	3.0/2.1			20/1	LTS-CLERKS	10	12			
12/15	1	EF A4-3	3	3.0/2.1			20/1	LTS-JUDGES	12	12			
12/17	1		20		3.0/2.1		20/1	LTS-JUDGES	14	12			
12/19	1		3	3.9/1.8			20/1	LTS-JUDGES	16	12			
10/21	1	AMU A4-1	3	3.9/1.8			20/1	LTS-CLERKS	18	12			
10/23	1		30		3.9/2.0		20/1	LTS-JUDGES	20	12			
-23 PRB	1		1	- 2.0			20/1	SPARE	22	12			
-27 PRB	1		1	- 2.0			20/1	SPARE	24	12			
-29 PRB	1		1	- 2.0			20/1	SPARE	26	12			
CONNECTED LOAD/PHASE				18.4	18.4	18.7	TC- DENOTES TIME CLOCK						
TOTAL CONNECTED LOAD				55.5	KVA	CONTROLLED LIGHTING CIRCUIT							
DEMAND FACTOR				1.0									
TOTAL DEMAND LOAD				55.5	KVA=	67	AMPS						

FIELD POSTED RECORD DRAWINGS[illegible]

45 44

PANEL "2C2A"										120/208 VOLTS, 3 PHASE, 4 WIRE 225 AMP MAIN LUGS		MIN AC: 10,000 MOUNTING: SURFACE	
CT NO	USE	CT BKR	PHASE A	PHASE B	PHASE C	CT BKR	AMP-POLE	USE	CT NO				
12.1	RECEPT-COULT 2071	1	20	0.6	1.0	20.1	RECEPT-LOBBY	2	12				
12.2	RECEPT-COULT 2071	1	20	0.8	1.2	20.1	COPIER	4	12				
12.3	RECEPT-COULT 2071	1	20	0.8	1.2	20.1	COPIER	6	12				
12.7	RECEPT-COULT 2071	1	20	1.2	1.2	20.1	RECEPT-BALUF	8	12				
12.9	RECEPT-COULT 2071	1	20	0.8	0.8	20.1	RECEPT-BALUF	10	12				
12.11	RECEPT-COULT 2071	1	20	0.8	1.2	20.1	RECEPT-COULF	12	12				
12.13	RECEPT-COULT 2080	1	20	0.6	1.0	20.1	RECEPT-INTERVIEW	14	12				
12.15	RECEPT-COULT 2080	1	20	0.8	1.2	20.1	RECEPT-OBSERV	16	12				
12.17	RECEPT-COULT 2080	1	20	1.2	1.0	20.1	RECEPT-COULF	18	12				
12.19	RECEPT-COULT 2080	1	20	0.8	1.2	20.1	COURTUM MONITORS	20	12				
12.21	RECEPT-COULT 2080	1	20	0.8	1.0	20.1	COURTUM MONITORS	22	12				
10.29	RECEPT-COULT 2087	1	20	0.6	1.0	20.1	ROLLING SHADE	24	10				
10.27	RECEPT-COULT 2087	1	20	0.8	1.0	20.1	ROLLING SHADE	26	10				
10.21	RECEPT-COULT 2087	1	20	0.8	1.0	20.1	RECEPT-OBSEV	28	-				
10.13	RECEPT-COULT 2087	1	20	1.0	-	20.1	SPACE	30	-				
10.33	RECEPT-COULT 2087	1	20	0.8	-	20.1	PRB	34	-				
10.35	RECEPT-COULT 2087	1	20	0.8	-	20.1	PRB	36	-				
10.37	RECEPT-COULT 2087	1	20	1.0	-	20.1	PRB	38	-				
10.39	ROLLING SHADE	1	20	0.5	-	20.1	PRB	40	-				
10.41	ROLLING SHADE	1	20	0.5	-	20.1	PRB	42	-				
CONNECTED LOAD/PHASE			11.2	10.5	10.9								
TOTAL CONNECTED LOAD			32.6	KVA									
DEMAND FACTOR			0.8										
TOTAL DEMAND LOAD			26.0	KVA= 72 AMPS									

PANEL "2C1E"

PANEL "2C2B"

PANEL "2C1F"



Certified by: Seth, Harushige Date: Dec 11, 2013
Unlimited Construction Services, Inc.


WIRE SIZE (AWG)		PANEL "2C3E"										WIRE SIZE (AWG)	
		120/208 VOLTS, 3 PHASE, 4 WIRE 225 AMP MAIN LUGS											
		MAN AGC 10,000 MOUNTING: SURFACE											
CKT	USE	CKT BRK	CONNECTED LOAD (KVA)	CKT BRK	USE	CKT	WIRE SIZE	CKT	USE	CKT BRK	CONNECTED LOAD (KVA)	CKT BRK	WIRE SIZE
NO		POL AMP	PHASE A	PHASE B	PHASE C	AMP		AMP		AMP	PHASE A	PHASE B	PHASE C
12	1 RECEPT-COULT 3035	1	20	1.0	0.8	20	1	20	1 RECEPT-RESTRANS	2	10	20	1
12	3 RECEPT-COULT 3035	1	20	0.8	1.2	20	1	20	1 RECEPT-MEDION CONF	4	10	20	1
12	5 RECEPT-COULT 3035	1	20			20	1	20	1 RECEPT-TRO SUPV	6	10	20	1
12	7 RECEPT-COULT 3035	1	20	1.2	1.0	20	1	20	1 SPARE	8	10	20	1
12	9 RECEPT-COULT 3035	1	20	0.8	0.8	20	1	20	1 MODULAR FLURN-ADULT	10	10	20	1
12	11 RECEPT-COULT 3035	1	20			20	1	20	1 MODULAR FLURN-ADULT	12	10	20	1
12	13 RECEPT-INTERVIEW	1	20	1.2	0.8	20	1	20	1 MODULAR FLURN-ADULT	14	10	20	1
12	15 RECEPT-INTERVIEW	1	20	1.2	0.8	20	1	20	1 MODULAR FLURN-ADULT	16	10	20	1
12	17 RECEPT-INTERVIEW	1	20			20	1	20	1 MODULAR FLURN-ADULT	18	10	20	1
12	19 RECEPT-ELEC/MECH	1	20	0.6	1.2	20	1	20	1 MODULAR FLURN-ADULT	20	10	20	1
12	21 RECEPT-ELECTRS COPIER	1	20	1.2	1.2	20	1	20	1 MODULAR FLURN-ADULT	22	10	20	1
12	23 RECEPT-SUPV	1	20			20	1	20	1 MODULAR FLURN-ADULT	24	10	20	1
12	25 RECEPT-SUPV	1	20	1.0	1.2	20	1	20	1 RECEPT-BALUF SUPV	26	10	20	1
12	27 RECEPT-SUPV	1	20	0.8	0.8	20	1	20	1 RECEPT-BALUF SUPV	28	10	20	1
12	29 RECEPT-SUPV	1	20			20	1	20	1 RECEPT-BALUF WK	30	10	20	1
12	31 RECEPT-SUPV	1	20	0.8	1.2	20	1	20	1 RECEPT-BREK	32	10	20	1
12	33 RECEPT-SUPV	1	20	0.8	1.0	20	1	20	1 RECEPT-BREK	34	10	20	1
12	35 RECEPT-COPY AREA	1	20			20	1	20	1 RECEPT-BREK	36	10	20	1
12	37 RECEPT-COPIER	1	20	1.2	1.0	20	1	20	1 SPARE	38	10	20	1
12	39 RECEPT-COPIER	1	20	1.2	1.0	20	1	20	1 SPARE	40	10	20	1
12	41 SPARE	1	20			20	1	20	1 SPARE	42	10	20	1
CONNECTED LOAD/PHASE			14.2		13.6		1.0	1.0		13.6			
TOTAL CONNECTED LOAD			41.4	KVA									
DEMAND FACTOR			0.8										
TOTAL DEMAND LOAD			33.0	KVA= 92 AMPS									

[illegible]

PANEL "2C3D"										120/208 VOLTS, 3 PHASE, 4 WIRE 225 AMP MAIN LUGS		MIN ALC: 10.000 MOUNTING: SURFACE	
WIRE SIZE (AWG)		CCT		CONNECTED LOAD (KVA)		CCT		WIRE SIZE (AWG)					
NO	USE	POLE	PHASE A	PHASE B	PHASE C	AMP	POLE	USE	NO				
12	1 MODULAR FURN-CLIENTS	1	20	1.0	0.8		20	1 MODULAR FURN-CLIENTS	2				
12	3 MODULAR FURN-CLIENTS	1	20		1.0	0.8	20	1 MODULAR FURN-CLIENTS	4				
12	5 MODULAR FURN-CLIENTS	1	20		1.0	0.8	20	1 MODULAR FURN-CLIENTS	6				
12	7 MODULAR FURN-CLIENTS	1	20	1.0	0.8		20	1 MODULAR FURN-CLIENTS	8				
12	9 MODULAR FURN-CLIENTS	1	20		0.8	0.8	20	1 MODULAR FURN-CLIENTS	10				
12	11 MODULAR FURN-CLIENTS	1	20			0.8	0.8	20	1 MODULAR FURN-CLIENTS	12			
12	13 MODULAR FURN-CLIENTS	1	20	0.8	0.8		20	1 MODULAR FURN-CLIENTS	14				
12	15 MODULAR FURN-CLIENTS	1	20		0.8	0.8	20	1 MODULAR FURN-CLIENTS	16				
12	17 MODULAR FURN-CLIENTS	1	20			0.8	0.8	20	1 MODULAR FURN-CLIENTS	18			
12	19 MODULAR FURN-CLIENTS	1	20	0.8	0.8		20	1 MODULAR FURN-CLIENTS	20				
12	21 MODULAR FURN-CLIENTS	1	20		0.8	0.8	20	1 MODULAR FURN-CLIENTS	22				
12	23 MODULAR FURN-CLIENTS	1	20			0.8	0.8	20	1 MODULAR FURN-CLIENTS	24			
12	25 MODULAR FURN-CLIENTS	1	20	1.2	1.0		20	1 SPARE	26				
12	27 MODULAR FURN-CLIENTS	1	20		1.2	1.0	20	1 SPARE	28				
12	29 MODULAR FURN-CLIENTS	1	20			1.2	1.0	20	1 SPARE	30			
12	31 MODULAR FURN-CLIENTS	1	20	1.2	1.0		20	1 SPARE	32				
12	33 MODULAR FURN-CLIENTS	1	20		0.8	1.0	20	1 SPARE	34				
12	35 MODULAR FURN-CLIENTS	1	20			0.8	1.0	20	1 SPARE	36			
12	37 MODULAR FURN-CLIENTS	1	20	0.8	—		—	1 PBB	38				
12	39 MODULAR FURN-CLIENTS	1	20		0.8	—	—	1 PBB	40				
—	41 SPARE	1	20			1.0	—	1 PBB	42				
CONNECTED LOAD/PHASE			12.0	11.4	11.8								
TOTAL CONNECTED LOAD			35.2	KVA									
DEMAND FACTOR			0.8										
TOTAL DEMAND LOAD			28.0	KVA= 78	AMPS								

FIELD POSTED RECORD DRAWINGS Certified by: Seth H. Hunschke Date: December 10, 2012
Unlimited Construction Services, Inc.

PANEL "4R1"										MIN A/C: 14,000 MOUNTING: SURFACE	
480 VOLTS, 3 PHASE, 3 WIRE										400 AMP MAIN LUGS	
480 VOLTS, 3 PHASE, 3 WIRE										400 AMP MAIN LUGS	
480 VOLTS, 3 PHASE, 3 WIRE										400 AMP MAIN LUGS	
480 VOLTS, 3 PHASE, 3 WIRE										400 AMP MAIN LUGS	
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480 VOLTS, 3 PHASE, 3 WIRE										400 AMP MAIN LUGS	
480 VOLTS, 3 PHASE, 3 WIRE										400 AMP MAIN LUGS	
480 VOLTS, 3 PHASE											

PANEL "4R2"										480 VOLTS, 3 PHASE, 3 WIRE		400 AMP MAIN LUGS				MIN A/C: 14,000 MOUNTING: SURFACE	
Circuit Breaker		USE	CONNECTED LOAD (KVA)		PHASE A		PHASE B		PHASE C		AMP/POLE		USE	Circuit Breaker			
NO	SIZE (AWG)															NO	SIZE (AWG)
21	2	SEC ELEV 1	3	18.0	2.1		18.0	2.1		18.0	2.1	15	4	12		2	12
23	2	SHUNT TRIP SPACE	1	100								1	4	12		2	12
25	2	PRIV ELEV 1	3			18.0						1	4	12		2	12
29	2	SHUNT TRIP SPACE	1	100	18.0							1	4	12		2	12
13	2	PRIV ELEV 2	3									1	4	12		2	12
17	2	SHUNT TRIP SPACE	1									1	4	12		2	12
21	2	PRIV ELEV 2	3	18.0	2.0							1	4	12		2	12
23	2	SHUNT TRIP SPACE	1	100								3	4	12		2	12
TOTAL CONNECTED LOAD			58.1		58.1							4		12			10
DEMAND FACTOR			174.3		KVA							4		12			10
TOTAL DEMAND LOAD			158		KVA=	190		AMPS				4		12			10
"LH" - DENOTES CIRCUIT BREAKER WITH LOOK "OFF" HANDLE CLIP																	

PANEL "2R1"										120/208 VOLTS, 3 PHASE, 4 WIRE		MIN A/C: 10,000	
										3P40 AMP MAIN BREAKER		MOUNTING: SURFACE	

PANEL "2R2"										120/208 VOLTS, 3 PHASE, 4 WIRE		MIN A/C: 10,000	
										3P40 AMP MAIN BREAKER		MOUNTING: SURFACE	

PANEL "21A"									
120/208 VOLTS, 3 PHASE, 4 WIRE									
100 AMP MAIN LUGS									
MIN A/C: 10,000									
MOUNTING: SURFACE									
SIZE (AWG)	CT	USE	CT	BRK	CONNECTED LOAD (KVA)	PHASE A	PHASE B	PHASE C	AMP/POLE
12 1	1	PATCH/SWITCH RACK	1	20	1.2	1.5	1.2	1.5	2
12 3	1	PATCH/SWITCH RACK	1	20	1.2	1.5	1.2	1.5	2
12 5	1	SHERIFF SERVER	1	20	1.2	1.5	1.2	1.5	2
12 7	1	SHERIFF SERVER	1	20	1.2	1.5	1.2	1.5	2
12 9	1	RECEIPT-BKBD	1	20	0.5	1.2	0.5	1.2	2
12 11	1	RECEIPT-BKBD	1	20	0.5	1.2	0.5	1.2	2
12 13	1	FM-200 PANEL	1	20	1.0	1.0	1.0	1.0	3
12 15	1	FSO	1	20	1.0	1.0	1.0	1.0	3
17	1	SPARE	1	20	4.9	4.2	4.2	4.2	3
TOTAL CONNECTED LOAD			13.3		13.3				
DEMAND FACTOR			0.8		0.8				
TOTAL DEMAND LOAD			10.6		10.6				
			KVA=		29	AMPS			

PANEL "21B"									
120/208 VOLTS, 3 PHASE, 4 WIRE									
100 AMP MAIN LUGS									
MIN A/C: 10,000									
MOUNTING: SURFACE									
SIZE (AWG)	CT	USE	CT	BRK	CONNECTED LOAD (KVA)	PHASE A	PHASE B	PHASE C	AMP/POLE
12 1	1	PATCH/SWITCH RACK	1	20	1.2	1.0	1.2	1.0	2
12 3	1	PATCH/SWITCH RACK	1	20	1.2	1.0	1.2	1.0	2
12 5	1	COURT AV RACK	1	20	1.5	1.0	1.5	1.0	2
12 7	1	COURT AV RACK	1	20	1.5	1.0	1.5	1.0	2
12 9	1	RECEIPT-BKBD	1	20	0.6	1.0	0.6	1.0	2
12 11	1	RECEIPT-BKBD	1	20	0.6	1.0	0.6	1.0	2
13	1	PRB	1	20	1.0	1.0	1.0	1.0	3
15	1	PRB	1	20	1.0	1.0	1.0	1.0	3
17	1	PRB	1	20	3.7	2.8	3.1	3.1	3
TOTAL CONNECTED LOAD			9.6		9.6				
DEMAND FACTOR			0.8		0.8				
TOTAL DEMAND LOAD			7.7		7.7				
			KVA=		21	AMPS			

PANEL "212A"									
120/208 VOLTS, 3 PHASE, 4 WIRE									
100 AMP MAIN LUGS									
MIN A/C: 10,000									
MOUNTING: SURFACE									
SIZE (AWG)	CT	USE	CT	BRK	CONNECTED LOAD (KVA)			CT	BRK
	NO		NO	AMP/POLE	PHASE A	PHASE B	PHASE C	NO	AMP/POLE
12 1	1	PATCH/SWITCH RACK	1	20	1.2	1.5		20	1
12 3	1	PATCH/SWITCH RACK	1	20				20	1
12 5	1	RECEIPT-BKBD	1	20		1.2	1.5	20	1
12 7	1	RECEIPT-BKBD	1	20	0.6	1.5		20	1
12 9	1	FM-200 PANEL	1	20			0.6	1.5	20
12 11	1	FSO	1	20		1.0	1.5	20	1
13	1	PRB	1	20	-	1.5		20	1
15	1	PRB	1	20		-	1.5	20	1
17	1	PRB	1	20			-	1.5	20
19	1	PRB	1	20	-	3.5		20	1
21	1	PRB	1	20		-	3.5	20	1
23	1	PRB	1	20			-	3.5	20
TOTAL CONNECTED LOAD					9.8	10.2	9.1	PANEL "212B"	
DEMAND FACTOR					29.1	KVA		24 6	
TOTAL DEMAND LOAD					23.3	KVA=	65	AMPS	